°SECTION 1

- DRAWINGS ARE PRELIMINARY AND NOT FOR CONSTRUCTION UNLESS TRUCTURAL ENGINEER'S STAMP IS AFEIVED TO DR
- ANY DISCREPANCIES IN THE DRAWINGS, NOTES AND SPECIFICATIONS, SHALL BE REPORTED TO OWNER'S REPRESENTATIVE FOR CLARIFICATION. THE CONTRACTOR SHALL VERIFY AND COORDINATE DIMENSIONS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION
- III. THE CONTRACTOR IS RESPONSIBLE FOR ALL BRACING AND SHORING DURING CONSTRUCTION
- IV. CONTRACTOR TO SUBMIT A REQUEST TO ENGINEER/ARCHITECT FOR ANY SUBSTITUTION OF MATERIALS OR PRODUCTS SPECIFIED ON THE DRAWINGS.
- V. STRUCTURAL DESIGN PER 2015 INTERNATIONAL BUILDING CODE.
- VI ALL CONSTRUCTION TO CONFORM TO 2015 IBC
- VII. THE FOLLOWING NOTES APPLY UNLESS SHOWN OTHERWISE
- VIII. THESE DRAWINGS HAVE BEEN PREPARED SOLELY FOR THE USE IN THE CONSTRUCTION OF A PROPOSED BUILDING TO WHICH THESE NOTES ARE ATTACHED. THE DRAWINGS SHALL NOT BE USED IN WHOLE OR IN PART, FOR FABRICATION OR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT THE WRITTEN CONSENT OF THE ENGINEER

°SECTION 2

I.	ROOF	LOADING

PITCHED ROOF: LIVE LOAD (SNOW): 35 PSF DEAD LOAD: 15 PSF TOTAL LOAD 50 PSF

FLOOR LOADING П. 40 PSF LIVE LOAD: DEAD LOAD: 12 PSF

TOTAL LOAD:

III. WIND LOADING: V = 115. EXPOSURE C

SEISMIC LOADING: SS = 0.6, S1 = 0.4

3 DESIGN SOIL PARAMETERS

1500 PSF BEARING PRESSURE ASSUMED WITH 45 PCF E.F.P. ACTIVE LATERAL EARTH PRESSURE IBC TABLE 1804.2 CLASS 4 MATERIAL(S)

52 PSF

SECTION 3 - CONCRETE

5/5/

Ğ GENERAL REQUIREMENTS

> STRUCTURAL CONCRETE FOR FOOTING SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2,500 PSI. CONCRETE FOR SLABS ON GRADE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF AND OF ANDE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF A,000 PSI AND A MAXIMUM WATER CEMENT RATIO OF 0.5. ALL OTHER CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIV STRENGTH OF 3,000 PSI. MINIMUM CEMENT CONTENT SHALL BE 5 SACKSICU. YD. MAXIMUM SIZE AGGREGATE SHALL BE 3/4*, SLUMP NOT TO EXCEED 4"

CAST IN PLACE CONCRETE

IF SOFT. SPONGY OR WET SOILS ARE ENCOUNTERED CONSTRUCTION IS TO BE STOPPED AND ENGINEER CONTACTED IMMEDIATELY

opyright CONCRETE FORM WORK TO BE OF ADEQUATE SIZE AND STRENGTH PROPERLY BRACED TO PREVENT SAGGING OR BULGING. PROTECT ALL CONCRETE FROM FREEZING TEMPERATURES. REFER TO DRAWING FOR DIMENSIONS OF CONCRETE MEMBERS AND SIZE AND LOCATION OF ALL REINFORCEMENT.

B. FOOTINGS

NO FOOTING SHALL BE PLACED ON DISTURBED SOIL (IF DISTURBED COMPACT SOIL IN 6" LIFTS TO 90% OF MAXIMUM DRY DENSITY PER ASTM D1557). FOOTINGS SHALL BE STEPPED DOWN ONE (1) VERTICALLY TO ONE AND ONE HALF (1-1/2) HORIZONTALLY.

C. FOUNDATION WALLS

REINFORCE PER DRAWINGS. DO NOT BACKEILL WALLS UNTIL MAIN REINFORCE PER DRAWINGS. DO NOI BACKFILL WALLS UNTIL MAIN FLOOR IS FRAMED AND SHEATHED AND CONCRETE HAS CURED A A MINIMUM OF 7 DAYS. USE HAND OPERATED COMPACTION EQUIPMENT ADJACENT TO NEWLY PLACED CONCRETE BASEMENT WALLS.

D. CONCRETE PADS AND THICKENED SLABS

REFER TO DRAWINGS AS TO SIZE AND REINFORCEMENT

E. CONCRETE SLABS

SLABS ON GRADE, AS NOTED ON THE DRAWINGS, TO BEAR ON 6" COMPACTED GRAVEL BASE. MINIMUM SLAB REINFORCEMENT TO BE #3 RE-BAR @ 18" o.c. BOTH WAYS, PLACED 1" CLEAR FROM TOP FACE (U.N.O. ON PLANS).

- F. FIREPLACE FOOTINGS AND CMU WALLS
- REFER TO DRAWINGS

G. REINFORCEMENT

REINFORCEMENT SHALL BE ASTM A615, GRADE 60 FOR #5 BARS AND LARGER, GRADE 40 FOR #3 & #4 BARS. ALL REBAR LAPPED 30 TIMES DIAMETER, REBAR AT FOOTINGS TO HAVE 3" CLEAR COVER OF CONCRETE (U.N.O. ON DRAWINGS). PROVIDE CORNER BARS WITH 18" LEGS AT THE CORNERS OF ALL WALLS AND FOOTINGS, SIZE AND PLACEMENT TO MATCH HORIZONTAL REINFORCEMENT.

H. °ANCHOR BOLTS

ANCHOR BOLTS TO BE ASTM A307, 1/2" DIA. x 10" EMBEDDED IN FOUNDATION WALLS @ 2'-8" o.c. (MAX) U.N.O. (SEE FOUNDATION PLAN FOR REQUIREMENTS AT SHEARWALLS), BOLTS TO BE WITHIN 1'-0" OF ENDS OF SILL PLATES (COORDINATE WITH GENERAL CONTRACTOR). MINIMUM OF TWO ANCHOR BOLTS PER SILL PLATE

ALL POSTS SUPPORTED BY ISOLATED FOOTINGS TO HAVE POST ANCHORS UNLESS BRACED IN STUD WALLS.

REFER TO DRAWINGS FOR HOLDDOWN REQUIREMENTS. INSTALL REQUIRED EMBEDDED ITEMS PER MANUFACTURER'S CATALOG TO ENGAGE HOLDDOWNS

I. CONSTRUCTION AND CRACK CONTROL JOINTS

ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS AND OTHER FOREIGN MATERIAL PRIOR TO PLACING ADJACENT DUST, CHIPS AND OTHER FOREION MATERIAL PRICE TO PLACING ADJACEN CONCRETE. CRACK CONTROL JOINTS IN SLABS SHALL HAVE A MAXIMUM SPACING OF 15-0' IN BOTH DIRECTIONS. THE CONTRACTOR SHALL SUBMIT THE DETAILS AND PROPOSED LOCATIONS OF CONSTRUCTION JOINTS AND CRACK CONTROL JOINTS FOR REVIEW BEFORE STARTING CONSTRUCTION

J. VAPOR BARRIER

VAPOR BARRIER TO BE 4 MIL POLYETHYLENE SHEET PLACED ON VAPOR BARRIER TO BE 4 MIL POLTE INTLEME SHEET PLACED ON UNDISTURBED SOIL. VAPOR BARRIER UNDER SLAB ON GRADE, IF REQUIRED, PLACED ON COMPACTED GRAVEL WITH 1" OF DAMP SAND BETWEEN POLYETHYLENE VAPOR BARRIER AND CONCRETE.

K. EMBEDDED ITEMS FOR HD TYPE HOLDDOWNS TO BE ASTM A307 HEX HEADED BOLT IN THE DIAMETER AS SPECIFIED BY THE MANUFACTURER FOR THE HD, ALL BOLTS TO HAVE 3" MIN, CONCRETE SIDE COVER FOR THE HD. ALL BOLTS TO HAVE 3" MIN. CONCRETE SIDE COVER. EMBEDDMENT DEPTHS ARE 15" FOR BOLTS UP TO AND INCLUDING 1" DIA., 24" DEPTH FOR BOLTS OVER 1". TYPICAL REINFORCEMENT TO PASS UNINTERRUPTED ALONGSIDE HOLDDOWN AS APPLICABLE. COUPLER NUTS MAY BE USED TO EXTEND THE HOLDDOWN ANCHOR THROUGH THE FLOOR PLATE TO THE SHEAR WALL CHORD.

III. °EPOXY ANCHORS

FPOXY GROUTED ANCHORS, IF USED, SHALL CONFORM TO HILTI HIT OR HILTLHVA EPOXY SYSTEM OR ENGINEER APPROVED FOULVALENT INSTALL PER MANUFACTURER'S INSTRUCTIONS. WIRE BRUSH AND

°SECTION 4 - REINFORCED CONCRETE MASONRY UNITS (CMU)

I. GENERAL REQUIREMENTS

- A HOLLOW CONCRETE MASONRY LINITS SHALL CONFORM TO ASTM C90 HOLLOW CONCRETE MASONRY TONITS SHALL CONFORM TO AST MIGBO, GRADE N WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI (MASONRY FM=1500 PSI), USE TYPE M MORTAR WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI, GROUT SHALL BE A PEA GRAVEL CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI AND A MINIMUM SLUMP OF 6 INCHES.
- B. INSTALL CMU OF SIZE AND ARCHITECTURAL TYPE SPECIFIED. REINFORCE PER DRAWINGS. SOLID GROUT ALL CELLS BELOW GRADE ALL REINFORCE PER DRAWINGS. ALL SELES BOLOW GRADE, ALL REINFORCED CELLS, AND AS SPECIFIED IN DRAWINGS. ALL 8° CMU AT FIREPLACES TO BE SOLID GROUTED. MAXIMUM HEIGHT FOR GROUT LIFTS TO BE 4'-0" UNLESS CLEAN OUTS ARE USED.
- C. MASONRY WALLS TO BE LAID IN RUNNING BOND. REINFORCE THE WALLS HORIZONTALLY WITH (2) #4 BAR IN AN 8" HIGH BOND BEAM AT EACH FLOOR AND ROOF LEVEL, AT THE TOP AND BOTTOM OF WALLS, AND @ 4'-0" o.c. (MAX) BETWEEN. REINFORCE THE WALLS VERTICALLY WITH (1) #5 @ 4'-0" o.c. (MAX) FOR FULL HEIGHT OF THE WALL WITH (1) #5 (g ++0 UC, (WKX) FOR FOLL REIGHT OF THE WALL, PLUS ONE #5 VERTICAL AT EACH JAMB, CORNER AND DISCONTINUOUS END (U.N.O. ON DRAWINGS). LAP REINFORCEMENT 40 BAR DIAMETERS AT SPLICES. REFER TO DRAWINGS FOR LINTEL DETAILS AT FIREPLACES.
- D. PROVIDE DOWELS WITH STANDARD HOOKS BETWEEN FOUNDATIONS AND ALL CMU, DOWELS DRILLED AFTER THE FACT ARE NOT ACCEPTABLE UNLESS APPROVED BY THE ENGINEER. SIZE AND SPACING OF DOWELS TO MATCH VERTICAL REINFORCEMENT OF CMU. DOWELS TO PROJECT A MINIMUM OF 2'-0" INTO CMU AND 17 BAR DIAMETERS INTO FOUNDATION (U.N.O.)

II. VENEER ANCHORAGE

PROVIDE VENEER ANCHORAGE PER IBC 3006(D)1. ANCHOR TIES TO BE NOT LESS THAN 9 GA. GALVANIZED WIRE OR 22 GA. BY 1" GALVANIZED SHEET METAL. ANCHOR TIES SHALL BE SPACED NOT MORE THAN 24" o.d AND SUPPORT NO MORE THAN 2 SQ. FEET OF VENEER. TIES SHALL BE PROVIDED TO HORIZONTAL JOINT REINFORCEMENT WIRE OF 9 GA. OR EQUIVALENT. JOINT REINFORCEMENT TO BE CONTINUOUS WITH BUTT SPLICES BETWEEN TIES

°SECTION 5 - FRAMING LUMBER

I. SAWN STRUCTURAL LUMBER

- A. SAWN LUMBER SHALL BE DOUGLAS FIR-LARCH (DF-L) NO. 2 OR BETTER FOR ALL 2 INCH AND 4 INCH NOMINAL LUMBER AND DF-L NO. 1 OR BETTER FOR 6 INCH NOMINAL AND LARGER STRUCTURAL MEMBERS (U.N.O.)
- WOOD BEARING ON OR INSTALLED WITHIN 1" OF MASONRY OR CONCRETE SHALL E PRESSURE TREATED WITH AN APPROVED PRESERVA STEEL PLATE WASHERS AT ALL BOLT HEADS AND NUTS BEARING ON WOOD.
- C. ALL FRAMING DETAILS SHALL BE IN ACCORDANCE WITH CHAPTER 23 OF THE 2006 EDITION LINEESS OTHERWISE NOTED ON THE DRAWINGS ALL FRAMING 2006 EDITION, UNLESS OTHERWISE NOTED ON THE DRAWINGS. ALL FRAMING NALING SHALL CONFORM TO TABLE 2304.9.1 OF THE IEC UNLESS OTHERWISE SHOWN. PROVIDE STEEL STRAPS AT PIPES IN STUD WALLS AS REQUIRED BY IEC CHAPTER 23. PLUMBING AND ELECTRICAL RUNS IN STUD WALLS SHALL CONFORM TO CHAPTER 23. BOLTS SHALL BE STANDARD MACHINE BOLTS (A307). ALL NALLS SHALL BE COMMON WIRE OR GALVANIZED BOX NALLS. IF PNEUMATIC NALLERS ARE TO BE USED. CONTRACTOR MUST SUBMIT A SCHEDULE OF NALLS DESIDED AS SUBSTITUTION TO THE ASCHITECT ODE DEVISION. DESIRED AS SUBSTITUTION TO THE ARCHITECT OR ENGINEER FOR REVIEW A CHANGE IN THE NUMBER OF NAILS OR A CLOSER NAIL SPACING MAY RE REOURED
- METAL HANGERS AND CONNECTORS SHALL BE FULLY NAILED OR BOLTED UNLESS OTHERWISE NOTED ON THE DRAWINGS. METAL HANGERS OR CONNECTORS SHOWN ON THE DRAWINGS SHALL BE MANUFACTURED BY SIMPSON COMPANY. METAL HANGERS OR CONNECTORS BY OTHER MANUFACTURES MAY B CONSIDERED WHERE LOAD CAPACITY AND DIMENSIONS ARE EQUAL OR BETTER ALL SUBSTITUTIONS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW
- PROVIDE SOLID BLOCKING BELOW ALL BEARING WALLS, PROVIDE SOLID VERTICAL BOTH SIDES. STITCH STUD BUNDLES TOGETHER WITH 16d COMMON @ 18" o.c. MAXIMUM (U.N.O.) WHERE FLOOR BEAMS ARE FRAMED FLUSH WITHIN FLOOF AND TOP FLANGE HANGERS ARE SPECIFIED. BEAMS ARE TO BE BLOCKED UP TO JOIST HEIGHT WITH FULL WIDTH DF-L SPACER AS REQUIRED
- STRUCTURAL GLUED-LAMINATED TIMBER

ALL GLUED-LAMINATED TIMBER SHALL BE COMBINATION 24F-V4 FOR SIMPLY SUPPORTED BEAMS, COMBINATION 24F-V8 FOR BEAMS CONTINUOUS OVER SUPPORTS, AND COMBINATION L2 FOR COLUMNS (U.N.O.) FABRICATION TO BE IN ACCORDANCE WITH AITC 117. PROVIDE WET-USE ADHESIVES. MAXIMUM MOISTURE CONTENT SHALL BE 15%. PROVIDE MILD STEEL PLATE WASHERS AT ALL BOLT HEAS AND NUTS TO PROVIDE MILD STEEF PLATE WASHERS AT ALL BOLT HEAS AND NUTS TO PROVIDE MILD STEEF PLATE BEARING ON OR WITHIN 1* OF MASONRY OR CONCRETE SHALL BE TREATED WITH AN APPROVED PRESERVATIVE. SEAL END GRAIN OF ALL EXTERIOR, EXPOSED BEAMS INCLUDING NON-LOAD BEARING ARCHITECTURAL BEAMS.

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WOOD "I" JOISTS AS MANUFACTURED BY THE TRUS JOIST MacMILLAN CORPORATION SHALL BE DESIGNED AND CERTIFIED BY MANUFACTURER TO SUPPORT LOADINGS AS SHOWN ON THE DRAWINGS. SUBSTITUTION OF PRODUCTS BY OTHER MANUFACTURERS REQUIRES APPROVAL OF ENGINEER OF RECORD. JOISTS SHALL BE ERECTED, INSTALLED, AND BRACED IN ACCORDANCE

IV. LAMINATED VENEER LUMBER (LVL)

WITH MANUFACTURER'S RECOMMENDATIONS.

PRODUCTS SPECIFIED HEREIN AS MUOR MELAND PUSHALL CONFORM TO THE PERFORMANCE CRITERIA OF LU AND PSL PRODUCTS AS MANUFACTURED BY TRUSS JOIST MacMILLAN AS MICROLLAM AND PARALLAM. SUBSTITUTES ARE ACCEPTABLE PROVIDED THEY HAVE THE SAME STRUCTURAL VALUES AS ML AND PL. ANY SUBSTITUTIONS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW.

V. WOOD SHEATHING

A ALL WOOD SHEATHING SHALL BE APA RATED EXPOSURE 1 PLYWOOD OR OSE WITH THICKNESS, VENEER GRADES AND SPAN RATING AS NOTED HEREIN OF ON DRAWINGS,

ROOF SHEATHING 5/8" WITH MINIMUM (40/20) SPAN RATING.

FLOOR SHEATHING 3/4" T&G APA SPAN RATED TO 24"

EXTERIOR WALL AND SHEAR WALL SHEATHING 7/16" WITH MINIMUM (24/0) SPAN RATING.

- B. ROOF AND FLOOR SHEATHING TO BE LAID UP WITH FACE GRAIN PERPENDICULAR TO SUPPORTS AND END JOINTS STAGGERED 4'-0" INSTALL ROOF SHEATHING WITH "SPACE AT ALL PANEL EDGES, NAIL ROOF SHEATHING WITH 8d @ 6" o.c. AT SUPPORTED PANEL AND 12" o.c. AT INTERMEDIATE FRAMING. FLOOR SHEATHING WITH 10d @ 6" o.c. AT SUPPORTED PANEL EDGES AND 10" o.c. FIELD, U.N.O. HOLES ARE NOT PERMITTED IN DIAPHRAGMS UNLESS REVIEWED BY ENGINEER.
- NAIL EXTERIOR WALL SHEATHING WITH 8d @ 6" o.c. EDGES AND 12" o.c. FIELD, U.N.O IN SHEARWALL SCHEDULE. OFFSET VERTICAL JOINTS 4-0" INSTALL WITH __" GAP AT BUTT ENDS.

VI. WOOD SHEARWALLS

- A. WHERE PLYWOOD PANELS ARE APPLIED TO BOTH SIDES OF SHEARWALL, PANEL IOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING MEMBERS SHALL BE 3" (NOMINAL) WIDE AND NAILS ON EACH SIDE SHALL BE
- ALLOWABLE SHEAR VALUES IN SHEARWALL TABLE ARE FOR DOUGLAS FIR FRAMING MEMBERS (GROUP II). NO SUBSTITUTION OF LESSER GROUPS WILL BE ALLOWED. FASTENERS EXPOSED TO WEATHER SHALL BE ZINC COATED BY HOT DIP GALVANIZING MECHANICALLY DEPOSITED, OR ELECTRO-DEPOSITED.

VII. PRF-MANUFACTURED WOOD TRUSSES

WOOD TRUSSES SHALL BE FACTORY ASSEMBLED USING STRESS RATED MATERIALS DESIGNED TO SUPPORT LOADING SHOWN ON DRAVINGS. INSTALL AND BRACE PER MANUFACTURER. MANUFACTURER IS RESPONSIBLE FOR REVIEWING ALL CONNECTIONS AND FRAMING IN TRUSSED ROOF SYSTEMS ABOVE PLATE HEIGHT FOR COMPLETENESS AND COMPATIBILITY WITH TRUSS DESIGNS. THIS INCLUDES ALL EAVE OVERHANGS AND OVER-FRAMES, SHOP DRAWINGS, DETAILS AND DESIGN CALCULATIONS OF TRUSSED ROOF SYSTEM MUST BE STAMPED BY A LICENSED CIVIL ENGINEER AND SUBMITTED TO ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION

SECTION 6 - STRUCTURAL STEEL AND MISCELLANEOUS METALS

STEEL SHALL CONFORM TO ASTM A992 UNLESS OTHERWISE NOTED. SQUARE OR RECTANGULAR STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE B (FY=46KSI). ALL WORK SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF (IP-446K3), ALL WORK SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS". SHOP DRAWINGS SHALL BE SUBMITTED FOR THE OWNER'S REPRESENTATIVES REVEW BEFORE COMMENCING FABRICATION. SHOP DRAWINGS SHA SHOW ALL WELDING WITH AWS A24 SYMBOLS. ALL WELDING SHALL BE DONE BY STRUCTURAL WELDING CODE, AWS D11. ALL FIELD WELDING TO BE ACCOMPLISHED BY AWS CERTIFIED WELDERS. ALL STEEL ANCHORS, TIES AND OTHER MEMBERS TO BE OMENDEDING TO BE COM ASCHARVE VALUE BE CHEVILDING THE AUTOMOTIONE INGS SHALL EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED. ALL MACHINE BOLTS SHALL BE ASTM A307 U.N.O. AND SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF LOCKING NUTS. ALL NUTS, BOLTS, WASHERS AND MISC. STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED

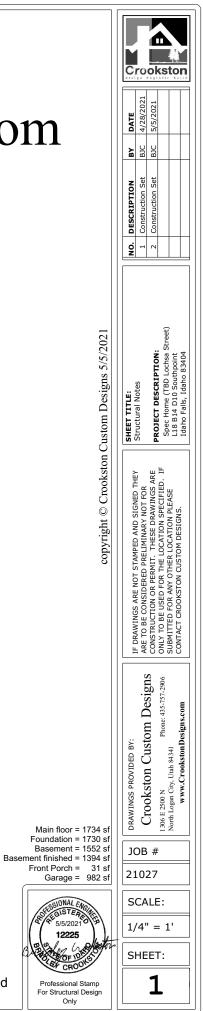
SECTION 7 - JOB SAFET

THE ENGINEER HAS NOT BEEN RETAINED NOR COMPENSATED TO PROVIDE DESIGN AND/OR CONSTRUCTION REVIEW SERVICES RELATED TO THE CONTRACTOR'S SAFETY PRECAUTIONS OR TO MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES FOR THE CONTRACTOR TO PERFORM HIS WORK. THE LINDERTAKING OF PERIODIC SITE THE CONTRACTOR TO PERFORM HIS WORK. THE UNDERTAINING OF PERIODIC STIE VISITS BY THE ENGINEER SHALL NOT BE CONSTRUED AS SUPERVISION OF ACTUAL CONSTRUCTION NOR MAKE HIM RESPONSIBLE FOR PROVIDING A SAFE PLACE FOR THE PERFORMANCE OF WORK BY THE CONTRACTOR, SUBCONTRACTORS, SUPPLIERS OR THEIR EMPLOYEES, OR FOR ACCESS, VISITS, USE WORK, OR OCCUPANCY BY ANY PERSON

	Layout Page Table
Sheet	Title
1	Structural Notes
2	Main Floor Plan
3	Foundation Plan
4	Basement Floor Plan
5	Front & Right Elevations
6	Back & Left Elevations
7	Sections & Details
8	Ledgend & Nailing Schedule
9	Framing Plans
10	Plot Plan

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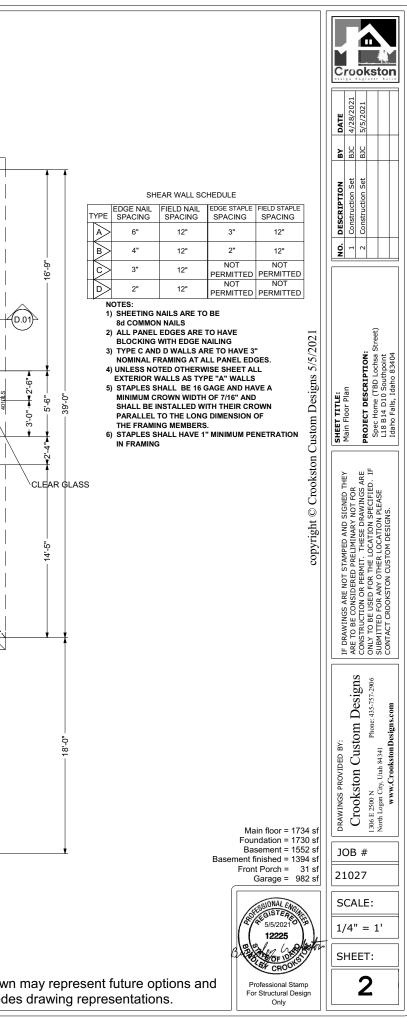


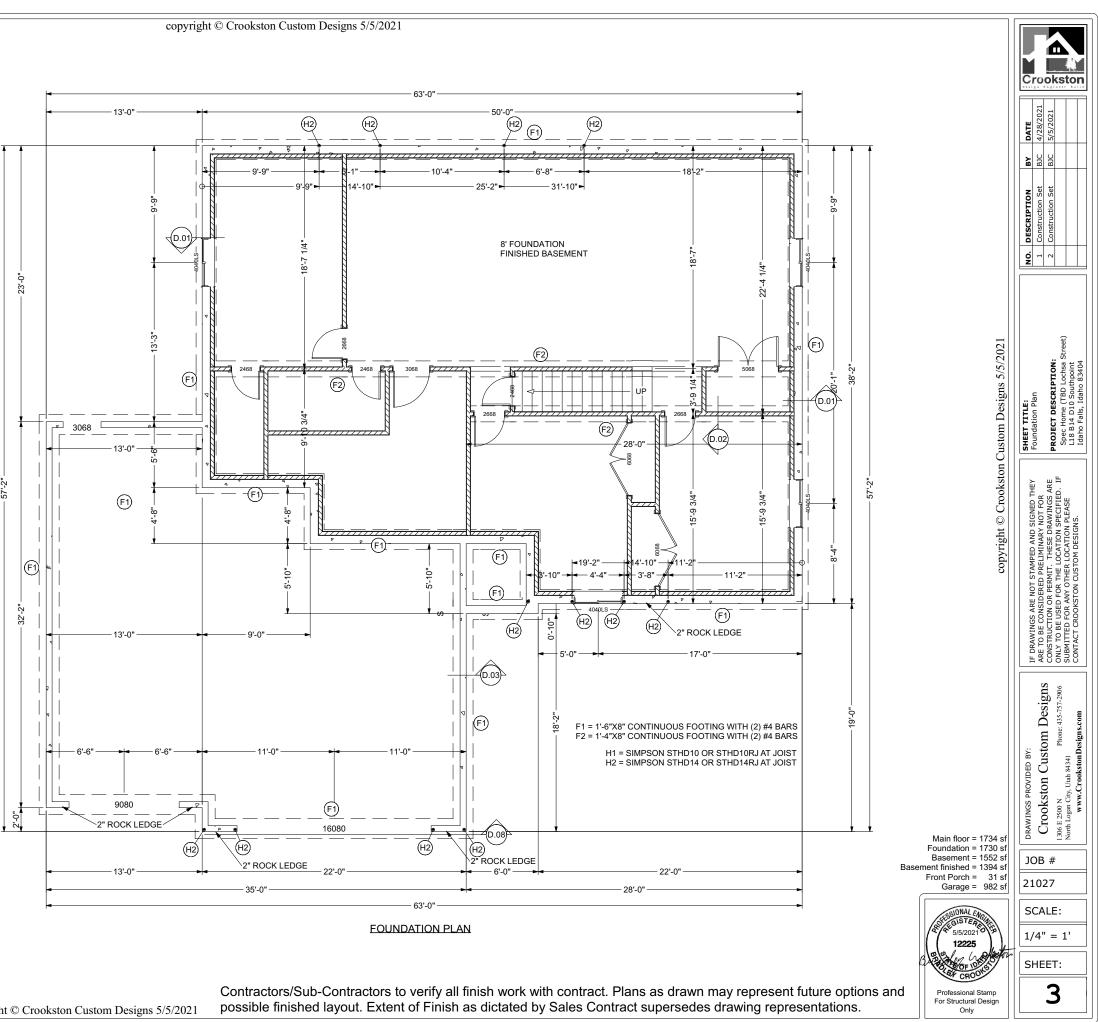
50'-0" 27'-5 1/2" 9'-4 1/2" - 13'-2 3'-11 1/2' (H2) (H2) (H2) (H2) MDF OBSCURE TEMPERED GLASS SHIPLAP TEMPERED GLASS WITH BLINDS THIS WALL CLEAR SHOWER OLASS 12'-8 1/2" LVP FLOORING TV OUTLET OVER FIRE PLACE MASTER BDRM 12'-8" X 15'-11" WITH BACKING LVP FLOORING SET FIREPLACE 1' OFF FLOOR NO HEARTH SHIPLAP ABOVE FIREPLACE 6 CARPET FLOORING 2) PENDENT LIGHTS LVP FLOORING FARM MDF SHIPLAP THIS WALL CUSTOM WOOD RANGE a∦.••∄ HOUSE SINK MDF SHIPLAP HOOD FROM KVO 3/12 VAULT (SD) INSIDE OF TRAY CEILING NO PLANT CARPET SHELF Section Flooring SQUARE TRAY CEILING ۳____ MDF SHIPLAP 13'-0' THIS WALL copyright © Crookston Custom Designs 5/5/202 LVP FLOORING (h) FULL WALL <u>FLO</u>ORING OPEN BELO -6-6 9'-3 1/2", 3/12 VAULT LVP RAILING FLOORING LVP IVP COISD 6" -FLOORING 9'-0" 4'x1'-8" WORKSTATION NO GRANITE 8'-5 1/ 1'-10" ò LYP FLOORING √ s⊃ (SD) þ BEDROOM #2 BEDROOM #1 11'-8" X 9'-2" MDF SHELF WITH HANGING ROD 7777 PROVIDE SELF CLOSING HINGE CARPET FLOORING 13'-0' CARPET FLOORING 8<u>18FX 1818FX 181</u>8FX īο 18" BENCH WITH j īc SHIPLAP BY KVO Ň <u></u> (H2)NON ACTIVE DORMER GARAGE 34'-4" X 28'-4" .0-.9 H1 = SIMPSON STHD10 OR STHD10RJ AT JOIST H2 = SIMPSON STHD14 OR STHD14RJ AT JOIST 1 (H2) (H2) LIVING AREA 1734 SQ FT 6'-6" 6'-6" 13'-0" 22'-0" 63'-0" Contractors/Sub-Contractors to verify all finish work with contract. Plans as drawn may represent future options and

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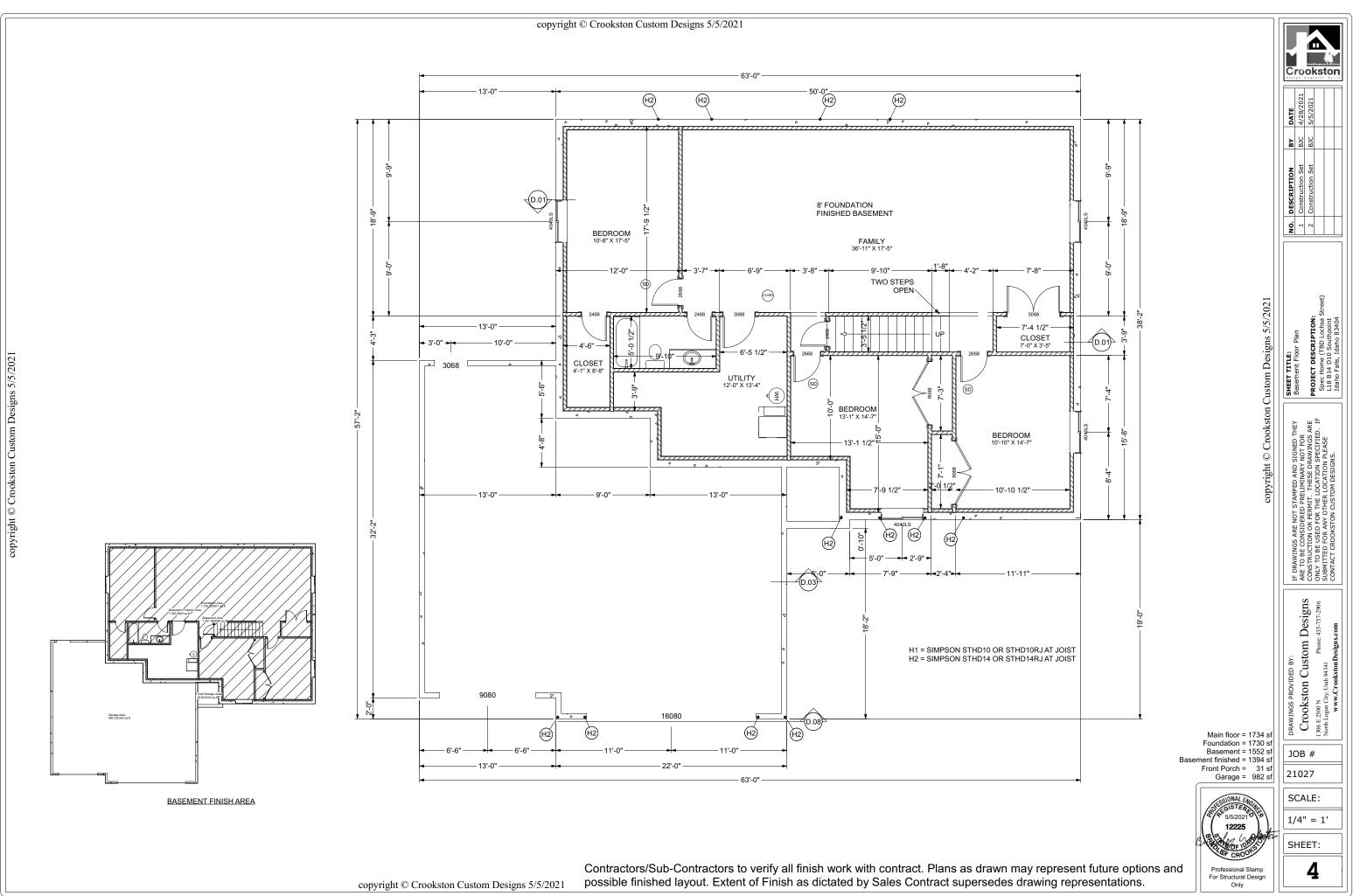
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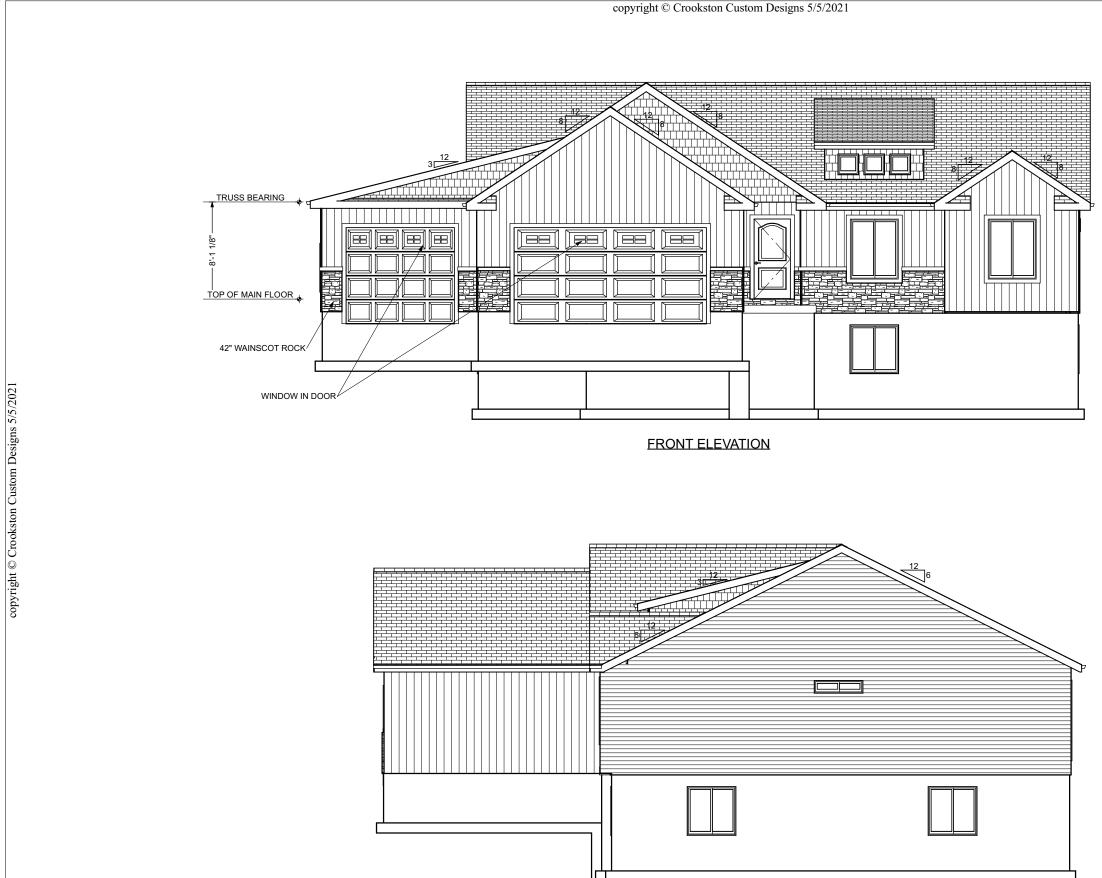
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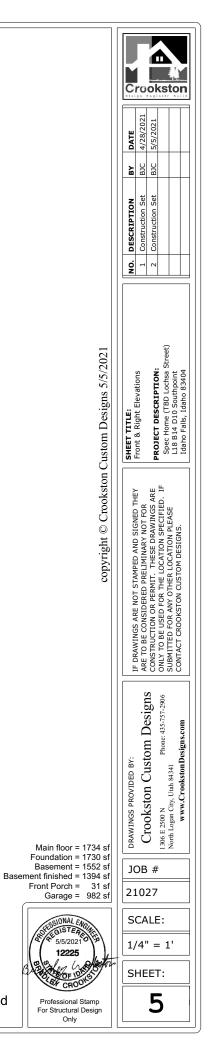


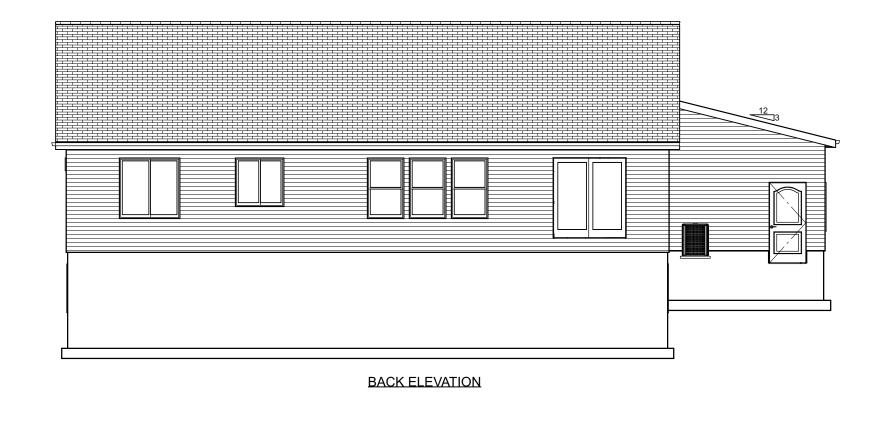


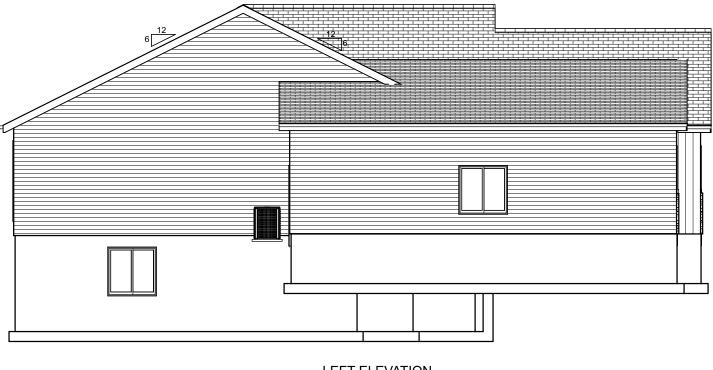
RIGHT ELEVATION

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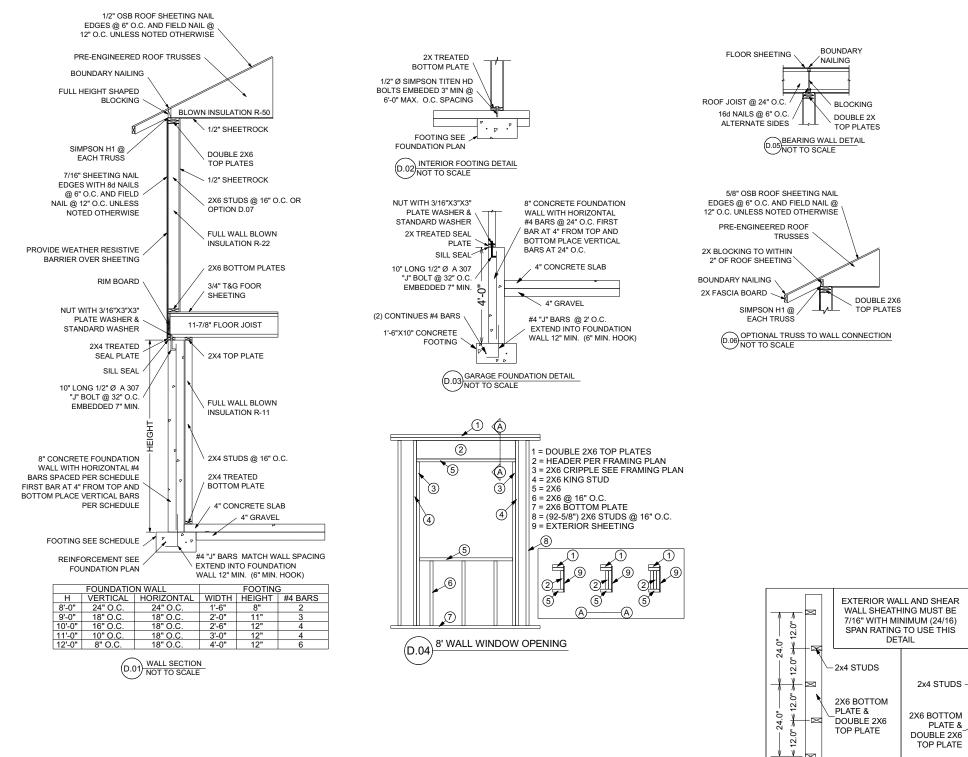




LEFT ELEVATION

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	NO. DESCRIPTION BY DATE 1 Construction Set BJC 4/28/202 2 Construction Set BJC 5/5/2021			
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copyright © Crookston Custom Designs 5/5/2021	SHEET TITLE: Back & Left Elevations PROJECT DESCRIPTION: Spec Home (TBD Lochsa Street) L18 B14 D10 Southpoint Labor Falls, Idaho 83404			
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Main floor = 4704 - 4	Crookston Custom Designs 0.00 Crookston Custom Designs 1.06 E 2500 N 1.06 E 250 N 1.06 E 25			
Main floor = 1734 sf oundation = 1730 sf Basement = 1552 sf nt finished = 1394 sf	JOB #			
ront Porch = 31 sf Garage = 982 sf	21027			
Stessional English	SCALE:			
12225 12225	1/4" = 1'			
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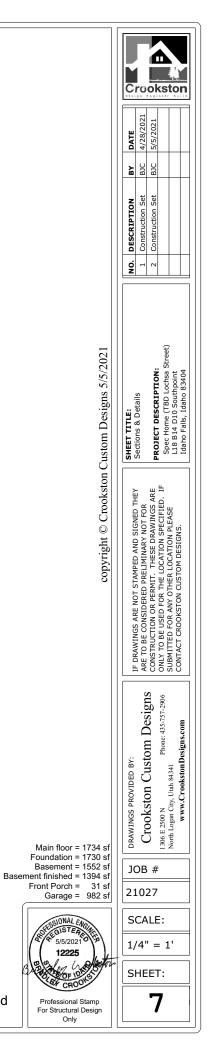
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D.07 OPTIONAL 2X4 STAGGERED STUDS

UP TO 9'-6" TALL WALL

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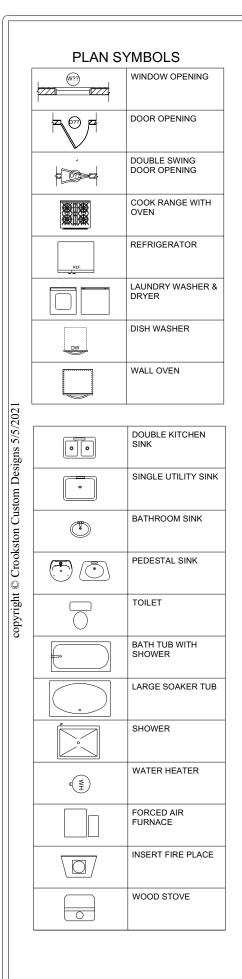
UP TO 10'-4" TALL WALL



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FASTENING SCHEDULE

CONNECTION



CONNECTION	FASTENING (a), (m)	LOCATION	
1. Joist to sill or girder	3 - 8d common (2-1/2" × 0.131")	toenail	
-	3 - 3" × 0.131" nails		
	3 - 3" 14 gage staples		
2. Bridging to joist	2 - 8d common (2-1/2" × 0.131")	toenail each end	
	2 - 3" × 0.131" nails		
	2 - 3" 14 gage staples	-	
3. 1" × 6" subfloor or less to each joist	2 - 8d common (2-1/2" × 0.131")	face nail	
4. Wider than 1" × 6" subfloor to each joist	3 - 8d common (2-1/2" × 0.131")	face nail	
5. 2" subfloor to joist or girder	2 - 16d common (3-1/2" × 0.162")	blind and face nail	
6. Sole plate to joist or blocking	16d (3-1/2" × 0.135 ") at 16" o.c.	typical face nail	
o. Sole plate to joist of blocking	3" × 0.131" nails at 8" o.c.	typical face nall	
Sole plate to joist or blocking at braced	3" 14 gage staples at 12" o.c.	-	
Wall panel		brood wall papels	
•	3- 16d (3-1/2" × 0.135") at 16" o.c.	braced wall panels	
	4 - 3" × 0.131" nails at 16" o.c.		
	4 - 3" 14 gage staples at 16" o.c.		
7. Top plate to stud	2 - 16d common (3-1/2" × 0.162")	end nail	
	3 - 3" × 0.131" nails		
	3 - 3" 14 gage staples		
8. Stud to sole plate	4 - 8d common (2-1/2" × 0.131")	toenail	
	4 - 3" × 0.131" nails		
	3 - 3" 14 gage staples		
	2 - 16d common (31/2" × 0.162")	end nail	
	3 - 3" × 0.131" nails		
	3 - 3" 14 gage staples		
9. Double studs	16d (3-1/2" × 0.135") at 24" o.c.	face nail	
	3" × 0.131" nail at 8" o.c.		
	3" 14 gage staple at 8" o.c.		
10. Double top plates	16d (3-1/2" × 0.135") at 16" o.c.	typical face nail	
	3" × 0.131" nail at 12" o.c.	51	
	3" 14 gage staple at 12" o.c.		
Double top plates	8 - 16d common (3-1/2" × 0.162")	lap splice	
	12 - 3" × 0.131" nails		
	12 - 3" 14 gage staples		
11. Blocking between joists or rafters to top	3 - 8d common (2-1/2" × 0.131")	toenail	
plate	3 - 3" × 0.131" nails	loenali	
plate	3 - 3" 14 gage staples	-	
		4 11	
12. Rim joist to top plate	8d (2-1/2" × 0.131") at 6" o.c.	toenail	
	3" × 0.131" nail at 6" o.c.		
	3" 14 gage staple at 6" o.c.	с н	
13. Top plates, laps and intersections	2 - 16d common (3-1/2" × 0.162")	face nail	
	3 - 3" × 0.131" nails		
	3 - 3" 14 gage staples		
14. Continuous header, two pieces	16d common (3-1/2" × 0.162")	16" o.c. along edge	
15. Ceiling joists to plate	3 - 8d common (2-1/2" × 0.131")	toenail	
	5 - 3" × 0.131" nails		
	5 - 3" 14 gage staples		
Continuous header to stud	4 - 8d common (2-1/2" × 0.131")	toenail	
17. Ceiling joists, laps over partitions	3 - 16d common (3-1/2" × 0.162") min.,	face nail	
(see Section 2308.10.4.1, Table 2308.10.4.1)	Table 2308.10.4.1		
	4 - 3" × 0.131" nails		
	4 - 3" 14 gage staples	1	
18. Ceiling joists to parallel rafters	3 - 16d common (3-1/2" × 0.162") minimum,	face nail	
(see Section 2308.10.4.1, Table 2308.10.4.1)	Table 2308.10.4.1		
· · · · · · · · · · · · · · · · · · ·	4 - 3" × 0.131" nails	1	
	4 - 3" 14 gage staples	1	
	3 - 8d common (2-1/2" × 0.131")	toenail	
19 Rafter to plate			
19. Rafter to plate (see Section 2308.10.1, Table 2308.10.1)	3 - 3" × 0.131" nails	lochail	

20 1" diagonal brace to each stud and plate		
20. 1" diagonal brace to each stud and plate	2 - 8d common (2-1/2" × 0.131")	face nail
	2 - 3″ × 0.131″ nails	
	3 - 3" 14 gage staples	-
21. 1" × 8" sheathing to each bearing	3 - 8d common (2-1/2" × 0.131")	face nail
22. Wider than 1" × 8" sheathing to each bearing	3 - 8d common (2-1/2" × 0.131")	face nail
23. Built-up corner studs	16d common (3-1/2" × 0.162")	24" o.c.
· · · · · · · · · · · · · · · · · · ·	3" × 0.131" nails	16" o.c.
	3" 14 gage staples	16" o.c.
24. Built-up girder and beams	20d common (4" × 0.192") 32" o.d	
2 Dant ap gradi and Doarno	3" × 0.131" nail at 24" o.c.	
	3" 14 gage staple at 24" o.c.	-
	2 - 20d common (4" × 0.192")	face nail at
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	-
25. 2" planks	16d common (3-1/2" × 0.162")	at each be
26. Collar tie to rafter	3 - 10d common (3" × 0.148")	face nail
26. Collar tie to ratter		tace hall
	4 - 3" × 0.131" nails 4 - 3" 14 gage staples	_
27. Jack rafter to hip	3 - 10d common (3" × 0.148")	toenail
	4 - 3" × 0.131" nails	
	4 - 3" 14 gage staples	-
	2 - 16d common (3-1/2" × 0.162")	face nail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
Roof rafter to 2-by ridge beam	2 - 16d common (3-1/2" × 0.162")	toenail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
	2 -16d common (3-1/2" × 0.162")	face nail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	-
29. Joist to band joist	3 - 16d common (3-1/2" × 0.162")	face nail
	4 - 3" × 0.131" nails	-
	4 - 3" 14 gage staples	_
30. Ledger strip	3 - 16d common (3-1/2" × 0.162") face nail at
	4 - 3" × 0.131" nails	<u>/</u>
	4 - 3" 14 gage staples	-
31. Wood structural panels and particleboard (b)	1/2" and less	6d (c), 1
Subfloor, roof and wall sheathing (to framing)		2-3/8" × 0.1
Subnool, fool and wan sheathing (to fraining)		2 0/0 00.
		1 2/4" 16 ~
	19/32" to3/4"	8d (d) or 6
	19/32" to3/4"	8d (d) or 60 2-3/8" × 0.2
	19/32" to3/4"	8d (d) or 60 2-3/8" × 0.2
Single floor (combination subfloor-underlayment to	19/32" to3/4"	8d (d) or 60 2-3/8" × 0.2
		8d (d) or 6d 2-3/8" × 0. 2" 16 gage
	7/8" to 1"	8d (d) or 6d 2-3/8" × 0.4 2" 16 gage 8d (c)
	7/8" to 1" 11/8" to 11/4"	8d (d) or 6d 2-3/8" × 0.1 2" 16 gage 8d (c) 10d (d) or 8
Single floor (combination subfloor-underlayment to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less	8d (d) or 6c 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e)
	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1"	8d (d) or 6 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 8d (e)
framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4"	8d (d) or 6 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 8d (e) 10d (d) or 8
	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less	8d (d) or 6 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 8d (e) 10d (d) or 8 6d (f)
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8"	8d (d) or 6 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 8d (e) 10d (d) or 8 6d (f) 8d (f)
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less	8d (d) or 6 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 4 6d (e) 8d (e) 10d (d) or 4 6d (f) 8d (f) No. 11 gag
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8"	8d (d) or 6d 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 4 6d (e) 8d (e) 10d (d) or 4 6d (f) 8d (f) No. 11 gag 6d commo
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8" 1/2"	8d (d) or 6 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 4 6d (e) 8d (e) 10d (d) or 4 6d (f) 8d (f) No. 11 gag 6d commo No. 16 gag
framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8"	8d (d) or 6c 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 8d (e) 10d (d) or 8 6d (f) 8d (f) No. 11 gag 6d common No. 16 gag
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8" 1/2"	8d (d) or 6c 2-3/8" × 0. 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 10d (d) or 8 6d (f) 8d (f) No. 11 gag No. 16 gag No. 11 gag
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8" 1/2"	10d (d) or 8 6d (e) 8d (e) 10d (d) or 8 6d (f) 8d (f) No. 11 gag 6d common No. 11 gag 8d common
framing) 32. Panel siding (to framing)	7/8" to 1" 11/8" to 11/4" 3/4" and less 7/8" to 1" 11/8" to 11/4" 1/2" or less 5/8" 1/2"	8d (d) or 6c 2-3/8" × 0.1 2" 16 gage 8d (c) 10d (d) or 8 6d (e) 8d (e) 10d (d) or 8 6d (f) 8d (f) No. 11 gag No. 16 gag No. 11 gag

FASTENING (a), (m)

a. Common or box nails are permitted to be used except where otherwise stated.
b. Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shar walls, refer to Section 2305. Nails for wall sheathing are permitted to be common, box or casing.
c. Common or deformed shank (6d - 2" × 0.113"; 8d - 21/2" × 0.131"; 10d - 3" × 0.148").
d. Common (6d - 2" × 0.113"; 8d - 21/2" × 0.131"; 10d - 3" × 0.148").
e. Deformed shank (6d - 2" × 0.113"; 8d - 21/2" × 0.131"; 10d - 3" × 0.148").
f. Corrosion-resistant siding (6d - 17/8" × 0.106"; 8d - 23/8" × 0.128") or casing (6d - 2" × 0.099"; 8d - 21/2" × 0.113") nail.
g. Fasteners spaced 3 inches on center at exterior edges and 6 inches on center at intermediate supports, when used as structural sheathing. Spacing shall be 6 inches on center on the edges and 12 inches on center at intermediate supports for nonstructural applications.
h. Corrosion-resistant roofing nails with 7/16-inch-diameter head and 1-1/2-inch length for 1/2-inch sheathing and 1-3/4-inch length for 25/32-inch sheathing.

 Corrosion-resistant staples with nominal 7/16-inch crown or 1-inch crown and 1-1/4-inc for 25/32-inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the marked).

j. Casing (1-1/2" × 0.080") or finish (1-1/2" × 0.072") nails spaced 6 inches on panel edg k. Panel supports at 24 inches. Casing or finish nails spaced 6 inches on panel edges, 12

I. For roof sheathing applications, 8d nails (21/2" × 0.113") are the minimum required for

m. Staples shall have a minimum crown width of 7/16 inch.

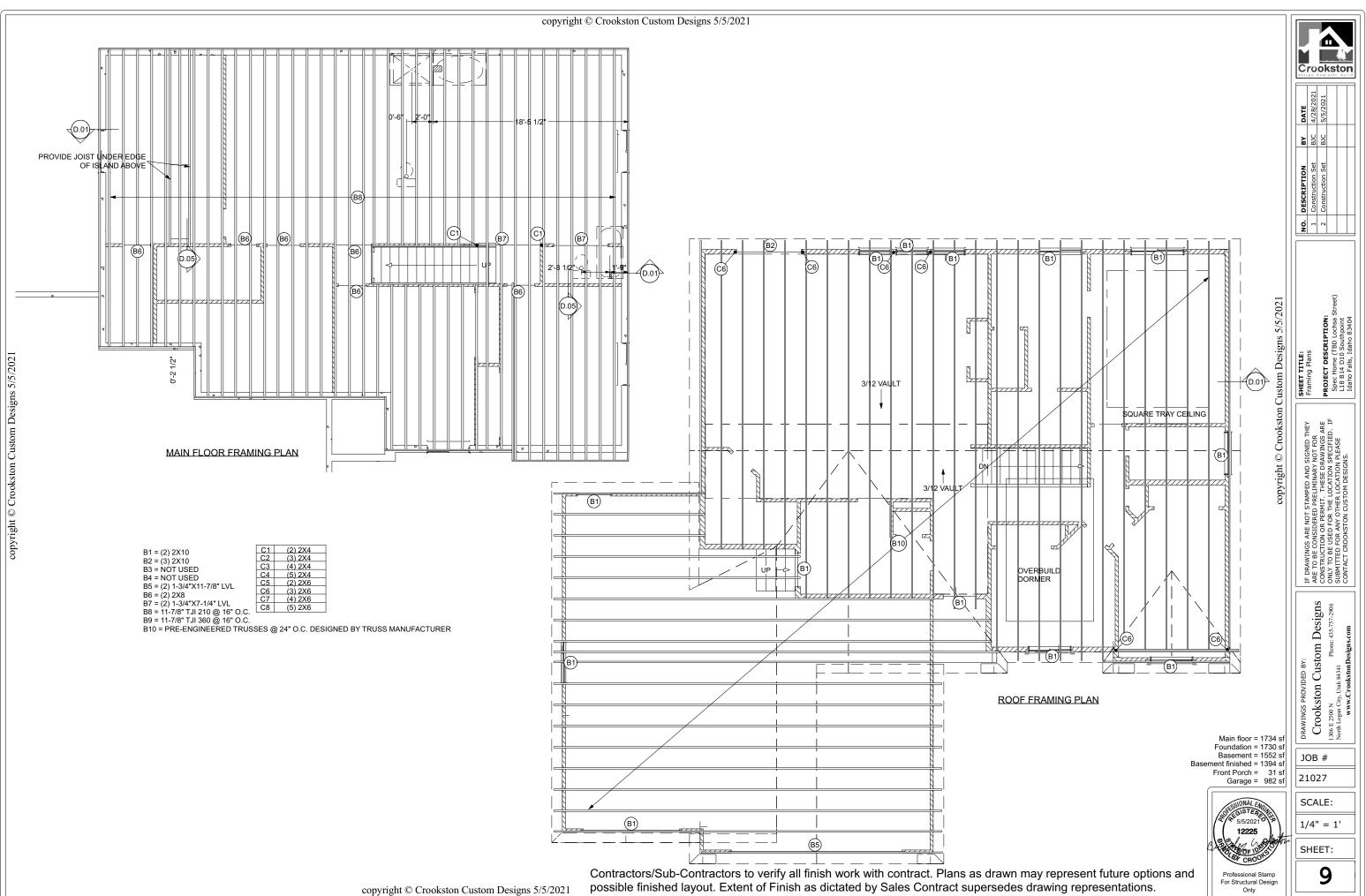
n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inche o. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports for s

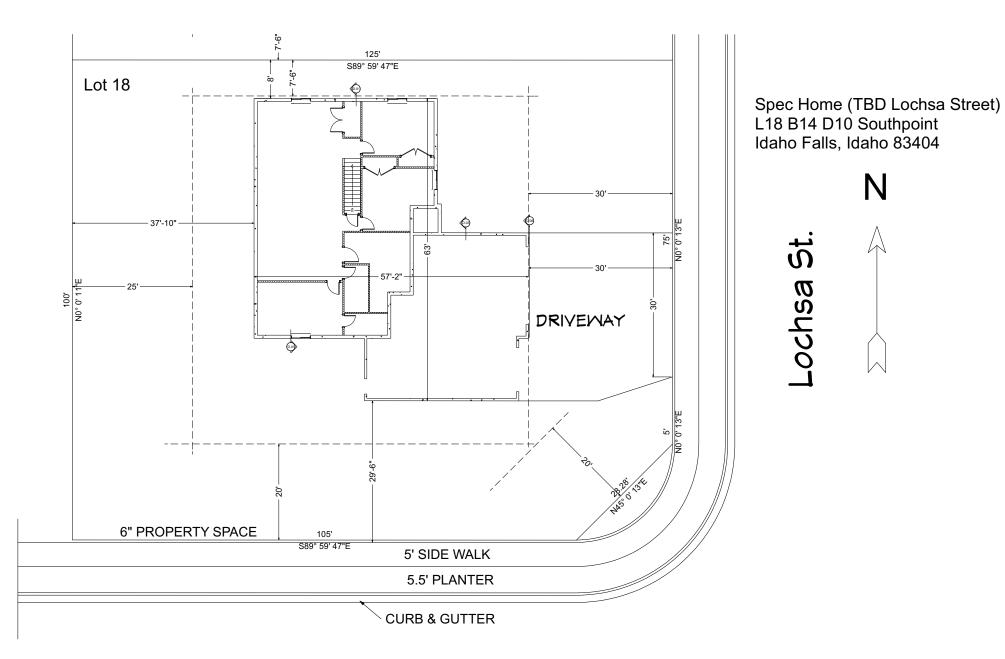
at edges, 6 inches at intermediate supports for roof sheathing.

p. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.

Contractors/Sub-Contractors to verify all finish work with contract. Plans as drawn may represent future options and possible finished layout. Extent of Finish as dictated by Sales Contract supersedes drawing representations.

LOCATION at top and bottom staggered on opposite sides at ends and at each splice bearing				No. DESCRIPTION BY DATE 1 Construction Set BJC 4/28/2021 2 Construction Set BJC 5/5/2021
		ustom Designs 5/5/2021		SHEET TITLE: Ledgend & Nailing Schedule PROJECT DESCRIPTION: Spec Home (TBD Lochsa Street) LI8 B14 D10 Southpoint Ldaho Falls, Idaho 83404
at each joist 0.113" nail (n) 6 gage (o) 6d (e) 0.113" nail (p) ge staple (p) or 8d (e) or 8d (e) age roofing pail (b)		copyright © Crookston Custom Designs 5/5/202		IF DRAWINGS ARE NOT STAMPED AND SIGNED THEY ARE TO BE CONSIDERED PRELIMINARY NOT FOR CONSTRUCTION OR PERMIT. THESE DRAWINGS ARE ONLY TO BE USED FOR THE LOCATION SPECIFIED. IF SUBMITTED FOR ANY OTHER LOCATION PLEASE CONTACT CROOKSTON CUSTOM DESIGNS.
age roofing nail (h) ton nail (2" × 0.113") age staple (i) age roofing nail (h) ton nail (2-1/2" × 0.131") age staple (i) ch length for 1/2-inch sheathing and 1-inch lengt long direction of the panel, unless otherwise res, 12 inches at intermediate supports. 2 inches at intermediate supports. r wood structural panels. as at intermediate supports. subfloor and wall sheathing and 3 inches on cent	 	Main floor = 1734 sf Foundation = 1730 sf Basement = 1552 sf ement finished = 1394 sf Front Porch = 31 sf Garage = 982 sf	2	DRAWINGS PROVIDED BY: DRAWINGS PROVIDED BY: # GCrookston Custom Designs North Logan City. Utah 84.341 North Logan City. Utah 84.341 www.CrookstonDesigns.com
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