- THE CONTRACTOR IS RESPONSIBLE FOR ALL BRACING AND SHORING DURING CONSTRUCTION.
- IV. CONTRACTOR TO SUBMIT A REQUEST TO ENGINEER/ARCHITECT FOR ANY SUBSTITUTION OF MATERIALS OR PRODUCTS SPECIFIED ON THE
- V. STRUCTURAL DESIGN PER 2015 INTERNATIONAL BUILDING CODE.
- VI. ALL CONSTRUCTION TO CONFORM TO 2015 IBC.
- VIII. THESE DRAWINGS HAVE BEEN PREPARED SOLELY FOR THE USE IN THE CONSTRUCTION OF A PROPOSED BUILDING TO WHICH THESE NOTES ARE ATTACHED. THE DRAWINGS SHALL NOT BE USED IN WHOLE OR IN PART, FOR FABRICATION OR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT THE WRITTEN CONSENT OF THE ENGINEER

I. ROOF LOADING

PITCHED ROOF LIVE LOAD (SNOW): DEAD LOAD:

II. FLOOR LOADING LIVE LOAD: DEAD LOAD: 12 PSF TOTAL LOAD: 52 PSF

III. WIND LOADING: 115 MPH. EXPOSURE O

IV. SEISMIC LOADING: SS = 0.6. S1 = 0.2

DESIGN SOIL PARAMETERS

1500 PSE REARING PRESSURE ASSUMED WITH 45 PCE E.F.P. ACTIVE LATERAL EARTH PRESSURE IBC TABLE 1804.2 CLASS 4 MATERIAL(S)

GENERAL REQUIREMENTS

STRUCTURAL CONCRETE FOR FOOTING SHALL HAVE A MINIMUM 28 DAY STRUCTURAL CONCRETE FOR FOOTING SHALL HAVE A MINIMUM 2I COMPRESSIVE STRENGTH OF 2,500 PSI. CONCRETE FOR SLABS ON GRADE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AND A MAXIMUM WATER CEMENT RATIO OF 0.5. ALL OTHER CONCRETE SHALL HAVE A MINIMUM 2B DAY COMPRESSIVE STRENGTH OF 3,000 PSI. MINIMUM CEMENT CONTENT SHALL BE SEASOCIAL UNITED AND AND AND ASSESSIVE STRENGTH OF 3,000 PSI. MINIMUM CEMENT CONTENT SHALL BE 5 SACKS/CU. YD. MAXIMUM SIZE AGGREGATE SHALL BE 3/4", SLUMP NOT TO EXCEED 4".

II CAST IN PLACE CONCRETE

IF SOFT, SPONGY OR WET SOILS ARE ENCOUNTERED CONSTRUCTION IS TO BE STOPPED AND ENGINEER CONTACTED IMMEDIATELY.

A. CONCRETE FORM WORK TO BE OF ADEQUATE SIZE AND STRENGTH PROPERLY BRACED TO PREVENT SAGGING OR BULGING. PROTECT ALL CONCRETE FROM FREEZING TEMPERATURES. REFER TO DRAWING FOR DIMENSIONS OF CONCRETE MEMBERS AND SIZE AND LOCATION

NO FOOTING SHALL BE PLACED ON DISTURBED SOIL (IF DISTURBED COMPACT SOIL IN 6" LIFTS TO 90% OF MAXIMUM DRY DENSITY PER ASTM D1557). FOOTINGS SHALL BE STEPPED DOWN ONE (1) VERTICALLY TO ONE AND ONE HALF (1-1/2) HORIZONTALLY

C. FOUNDATION WALLS

REINFORCE PER DRAWINGS. DO NOT BACKFILL WALLS UNTIL MAIN FLOOR IS FRAMED AND SHEATHED AND CONCRETE HAS CURED A A MINIMUM OF 7 DAYS. USE HAND OPERATED COMPACTION EQUIPMENT ADJACENT TO NEWLY PLACED CONCRETE BASEMENT WALLS.

D. CONCRETE PADS AND THICKENED SLABS

SLABS ON GRADE, AS NOTED ON THE DRAWINGS, TO BEAR ON 6" COMPACTED GRAVEL BASE. MINIMUM SLAB REINFORCEMENT TO BE #3 RE-BAR @ 18" o.c. BOTH WAYS, PLACED 1" CLEAR FROM TOP FACE

REFER TO DRAWINGS AS TO SIZE AND REINFORCEMENT

F. FIREPLACE FOOTINGS AND CMU WALLS

G. REINFORCEMENT

REINFORCEMENT SHALL BE ASTM A615, GRADE 60 FOR #5 BARS AND LARGER, GRADE 40 FOR #3 & #4 BARS. ALL REBAR LAPPED 30 TIMES DANCER, GROBE TO WARD AN BORNS, ALL REDAY DEFFECT OF THIS STANDARD TO THE STANDARD TO THE STANDARD THE STANDA

ANCHOR BOLTS TO BE ASTM A307, 1/2" DIA: x 10" EMBEDDED IN FOUNDATION WALLS @ 2'-8" o.c. (MAX) II N.O. (SEE FOUNDATION PLAN FOR REQUIREMENTS AT SHEARWALLS). BOLTS TO BE WITHIN 1-0" OF ENDS OF SILL PLATES (COORDINATE WITH GENERAL CONTRACTOR), MINIMUM OF TWO ANCHOR BOLTS PER SILL PLATE

ALL POSTS SUPPORTED BY ISOLATED FOOTINGS TO HAVE POST ANCHORS UNLESS BRACED IN STUD WALLS.

REFER TO DRAWINGS FOR HOLDDOWN REQUIREMENTS. INSTALL REQUIRED EMBEDDED ITEMS PER MANUFACTURER'S CATALOG TO ENGAGE HOLDDOWNS.

CONSTRUCTION AND CRACK CONTROL JOINTS

ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS AND OTHER FOREIGN MATERIAL PRIOR TO PLACING ADJACENT CONCRETE. CRACK CONTROL JOINTS IN SLABS SHALL HAVE A MAXIMUM SPACING OF 15'-0" IN BOTH DIRECTIONS. THE CONTRACTOR SHALL SUBMIT THE DETAILS AND PROPOSED LOCATIONS OF CONSTRUCTION JOINTS AND CRACK CONTROL JOINTS FOR REVIEW BEFORE STARTING CONSTRUCTION

J VAPOR BARRIER

VAPOR BARRIER TO BE 4 MIL POLYETHYLENE SHEET PLACED ON DAMP SAND BETWEEN POLYETHYLENE VAPOR BARRIER AND CONCRETE

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K. EMBEDDED ITEMS FOR HD TYPE HOLDDOWNS TO BE ASTM A307 HEX HEADED BOLT IN THE DIAMETER AS SPECIFIED BY THE MANUFACTURER FOR THE HD. ALL BOLTS TO HAVE 3" MIN. CONCRETE SIDE COVER FOR THE HJ. ALL BULTS 10 FARE 3" MIN. CONCRETE SIDE COVER.

BMBEDDMENT DEPTHS ARE 15" FOR BOLTS UP TO AND INCLUDING 1" DIA.,
24" DEPTH FOR BOLTS OVER 1". TYPICAL REINFORCEMENT TO PASS
UNINITERRUPTED ALONGSIDE HOLDDOWN AS APPLICABLE. COUPLER NUTS
MAY BE USED TO EXTEND THE HOLDDOWN ANCHOR THROUGH THE FLOOR PLATE TO THE SHEAR WALL CHORD.

III. °EPOXY ANCHORS

EPOXY GROUTED ANCHORS, IF USED, SHALL CONFORM TO HILTI HIT

°SECTION 4 - REINFORCED CONCRETE MASONRY UNITS (CMU)

- HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI (MASONRY PM=1500 PSI). USE TYPE M MORTAR WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. GROUT SHALL BE A PEA GRAVEL CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI AND A MINIMUM SLUMP OF 6 INCHES.
- INSTALL CMU OF SIZE AND ARCHITECTURAL TYPE SPECIFIED.
 REINFORCE PER DRAWINGS. SOLID GROUT ALL CELLS BELOW GRADE,
 ALL REINFORCED CELLS, AND AS SPECIFIED IN DRAWINGS. ALL & CMU
 AT FIREPLACES TO BE SOLID GROUTED. MAXIMUM HEIGHT FOR GROUT
 LIFTS TO BE 4'-0" UNLESS CLEAN OUTS ARE USED.
- C. MASONRY WALLS TO BE LAID IN RUNNING BOND. REINFORCE THE WALLS HORIZONTALLY WITH (2) #4 BAR IN AN 8" HIGH BOND BEAM AT EACH FLOOR AND ROOF LEVEL. AT THE TOP AND BOTTOM OF WALLS. AND FLOOR AND ROOF LEVEL, AT THE TOP AND BOTTOM OF WALLS, AND @ 4-0° o.c. (MAX) BETWEEN. REINFORCE THE WALLS VERTICALLY WITH (1) #5 @ 4-0° o.c. (MAX) FOR FULL HEIGHT OF THE WALL, PLUS ONE #5 VERTICAL AT EACH JAMB, CORNER AND DISCONTINUOUS END (U.N.O. ON DRAWINGS). LAP REINFORCEMENT 40 BAR DIAMETERS AT SPLICES. REFER TO DRAWINGS FOR LINTEL DETAILS AT FIREPLACES.
- D. PROVIDE DOWELS WITH STANDARD HOOKS BETWEEN FOUNDATIONS AND ALL CMU, DOWELS DRILLED AFTER THE FACT ARE NOT ACCEPTABLE LINESS APPROVED BY THE ENGINEER. SIZE AND SPACING OF DOWELS TO MATCH VERTICAL REINFORCEMENT OF CMU. DOWELS TO PROJECT A MINIMUM OF 2'-0" INTO CMU AND 17 BAR DIAMETERS INTO FOUNDATION

Number

4

6

8

9

PROVIDE VENEER ANCHORAGE PER IBC 3006(D)1. ANCHOR TIES TO BE NOT LESS THAN 9 GA. GAI VANIZED WIRE OR 22 GA. BY 1" GAI VANIZED SHEET METAL. ANCHOR TIES SHALL BE SPACED NOT MORE THAN 24" o. AND SUPPORT NO MORE THAN 2 SQ. FEET OF VENEER. TIES SHALL BE PROVIDED TO HORIZONTAL JOINT REINFORCEMENT WIRE OF 9 GA. OR EQUIVALENT. JOINT REINFORCEMENT WIRE OF 9 GA. OR SPLICES BETWEEN TIES.

Page Table

Title

Cover Page

Main Floor Plan

Foundation/Basement Plan

Basement Floor Plan

Front & Right Elevations

Back & Left Elevations

Sections & Details

Ledgend & Nailing Schedule

Framing Plans

Plot Plan

°SECTION 5 - FRAMING LUMBER

SAWN STRUCTURAL LUMBER

- A. SAWN LUMBER SHALL BE DOUGLAS FIR-LARCH (DF-L) NO. 2 OR BETTER FOR ALL 2 INCH AND 4 INCH NOMINAL LUMBER AND DF-L NO. 1 OR BETTER FOR 6 INCH NOMINAL AND LARGER STRUCTURAL MEMBERS (U.N.O.)
- B. WOOD BEARING ON OR INSTALLED WITHIN 1" OF MASONRY OR CONCRETE SHALL SSURE TREATED WITH AN APPROVED PRESERVATIVE PROVIDE MILE STEEL PLATE WASHERS AT ALL BOLT HEADS AND NUTS BEARING ON WOOD.
- ALL FRAMING DETAILS SHALL BE IN ACCORDANCE WITH CHAPTER 23 OF THE 2006 EDITION, UNLESS OTHERWISE NOTED ON THE DRAWINGS. ALL FRAMING NAILING SHALL CONFORM TO TABLE 2304.9.1 OF THE IBC UNLESS OTHERWISE SHOWN. PROVIDE STEEL STRAPS AT PIPES IN STUD WALLS AS REQUIRED BY IBC CHAPTER 23. PLUMBING AND ELECTRICAL RUNS IN STUD WALLS SHAL IBC CHAPTER 23. PLUMBING AND ELECTRICAL RUNS IN STUD WALLS SHALL OCNFORM TO CHAPTER 23. BOLTS SHALL BE STANDARD MACHINE BOLTS (A307). ALL NAILS SHALL BE COMMON WIRE OR GALVANIZED BOX NAILS. IF PNEUMATIC NAILERS ARE TO BE USED, CONTRACTOR MUST SUBMIT A SCHEDULE OF NAILS DESIRED AS SUBSTITUTION TO THE ARCHITECT OR ENGINEER FOR REVIEW. A CHANGE IN THE NUMBER OF NAILS OR A CLOSER NAIL SPACING MAY
- D. METAL HANGERS AND CONNECTORS SHALL BE FULLY NAILED OR BOLTED UNLESS OTHERWISE NOTED ON THE DRAWINGS. METAL HANGERS OR CONNECTORS SHOWN ON THE DRAWINGS SHALL BE MANUFACTURED BY SIMPSON COMPANY. METAL HANGERS OR CONNECTORS BY OTHER MANUFACTURES OF ANY BE CONSIDERED WHERE LOAD CAPACITY AND DIMENSIONS ARE EQUAL OR BETTER. ALL SUBSTITUTIONS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW.
- PROVIDE SOLID BLOCKING BELOW ALL BEARING WALLS. PROVIDE SOLID VERTICAL BLOCKING IN FLOOR SPACE TO MATCH STUD BUNDLE OR SOLID COLUMN ABOVE AND BELOW. VERTICAL BLOCKING AT WOOD "I" JOISTS SHALL BE 1/16" LONGER THAN JOIST IS DEEP. MINIMUM POST TO BE TWO 2x STUDS BEARING AT EACH END OF HEADER U.N.O. FOR BEAMS FRAMING PERPENDICULAR TO BEARING WALLS END OF HEADER U.N.O. FOR BEAMS FRAMING PERPENDICULAR TO BEARING WAL PROVIDE FULL WIDTH BEAM POCKET WITH FILLER AS REQUIRED AND KING STUD BOTH SIDES. STITCH STUD BUNDLES TOGETHER WITH 16d COMMON @ 18° o.c. MAXIMUM (U.N.O.) WHERE FLOOR BEAMS ARE FRAMED FLUSH WITHIN FLOOR AND TOP FLANGE HANGERS ARE SPECIFIED, BEAMS ARE TO BE BLOCKED UP TO JOIST HEIGHT WITH FULL WIDTH DF-L SPACER AS REQUIRED.

II. STRUCTURAL GLUED-LAMINATED TIMBER

ALL GLUED-LAMINATED TIMBER SHALL BE COMBINATION 24F-V4 FOR SIMPLY SUPPORTED BEAMS, COMBINATION 24F-V9 FOR BEAMS CONTINUOUS OVER SUPPORTS, AND COMBINATION 12 FOR COLUMNS (U.N.O.) FABRICATION TO BE IN ACCORDANCE WITH AITC 117. PROVIDE WET-USE ADHESIVES. MAXIMUM MOSITURE CONTENT SHALL BE 15%, PROVIDE MILD STEEL PLATE WASHERS AT ALL BOLT HEADS AND NUTS BEARING ON WOOD. WOOD BEARING ON OR WITHIN 1" OF MASONRY OR CONCRETE SHALL BE TREATED WITH AN APPROVED PRESERVATIVE. SEALEND GRAIN OF ALL EXTERIOR. EXPOSED BEAMS INCLUDING NON-LOAD BEARING ARCHITECTURAL BEAMS

III. MANUFACTURED WOOD "I" JOISTS

WOOD "I" JOISTS AS MANUFACTURED BY THE TRUS JOIST MacMILLAN CORPORATION SHALL BE DESIGNED AND CERTIFIED BY MANUFACTURER TO SUPPORT LOADINGS AS SHOWN ON THE DRAWINGS. SUBSTITUTION OF PRODUCTS BY OTHER MANUFACTURERS REQUIRES APPROVAL OF ENGINEER OF RECORD. JOISTS SHALL BE ERECTED, INSTALLED, AND BRACED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS

PRODUCTS SPECIFIED HEREIN AS ML OR M=L AND PL SHALL CONFORM TO THE PERFORMANCE CRITERIA OF LVL AND PSL PRODUCTS AS MANUFACTURED BY TRUSS JOIST MacMILLAN AS MICROLLAM AND PARALLAM. SUBSTITUTES ARE ACCEPTABLE PROVIDED THEY HAVE THE SAME STRUCTURAL VALUES AS ML AND PL ANY SUBSTITUTIONS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW

V WOOD SHEATHING

ALL WOOD SHEATHING SHALL BE APA RATED EXPOSURE 1 PLYWOOD OR OSE WITH THICKNESS, VENEER GRADES AND SPAN RATING AS NOTED HEREIN OR ON DRAWINGS,

ROOF SHEATHING 5/8" WITH MINIMUM (40/20) SPAN RATING.

ELOOR SHEATHING 3/4" T&G APA SPAN RATED TO 24

EXTERIOR WALL AND SHEAR WALL SHEATHING 7/16" WITH MINIMUM (24/0) SPAN RATING

- B. ROOF AND FLOOR SHEATHING TO BE LAID UP WITH FACE GRAIN PERPENDICULAR TO SUPPORTS AND END JOINTS STAGGERED 4'-0" INSTALL ROOF SHEATHING WITH "SPACE AT ALL PANEL EDGES. NAIL ROOF SHEATHING WITH 8d @ 6" o.c. AT SUPPORTED PANEL AND 12" o.c. AT INTERMEDIATE FRAMING, FLOOR SHEATHING WITH 8d @ 6" o.c. WITH 10d @ 6" o.c. AT SUPPORTED PANEL EDGES AND 10" o.c. FIELD, UN.O. HOLES ARE NOT PERMITTED IN DIAPHRAGMS UNLESS REVIEWED BY ENGINEER.
- C. NAIL EXTERIOR WALL SHEATHING WITH 8d @ 6" o.c. EDGES AND 12" o.c. FIELD, U.N.O. IN SHEARWALL SCHEDULE. OFFSET VERTICAL JOINTS 4'-0" INSTALL WITH _" GAP AT BUTT ENDS.

VI. WOOD SHEARWALLS

- A. WHERE PLYWOOD PANELS ARE APPLIED TO BOTH SIDES OF SHEARWALL, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING MEMBERS SHALL BE 3" (NOMINAL) WIDE AND NAILS ON EACH SIDE SHALL BE
- B. ALLOWABLE SHEAR VALUES IN SHEARWALL TABLE ARE FOR DOUGLAS FIR FRAMING MEMBERS (GROUP II), NO SUBSTITUTION OF LESSER GROUPS WILL BE ALLOWED. FASTENERS EXPOSED TO WEATHER SHALL BE ZINC COATED BY HOT DIP GALVANIZING, MECHANICALLY DEPOSITED, OR ELECTRO-DEPOSITED.

WOOD TRUSSES SHALL BE FACTORY ASSEMBLED USING STRESS RATED MATERIALS DESIGNED TO SUPPORT LOADING SHOWN ON DRAWINGS. INSTALL AND BRACE PER MANUFACTURER MANUFACTURER IS RESPONSIBLE FOR REVIEWING ALL CONNECTIONS AND FRAMING IN TRUSSED ROOF SYSTEMS ABOVE PLATE HEIGHT FOR COMPLETENESS AND COMPATIBILITY WITH TRUSS DESIGNS. THIS INCLUDES ALL EAVE OVERHANGS AND OVER-FRAMES. SHOP DRAWINGS, DETAILS AND DESIGN CALCULATIONS OF TRUSSED ROOF SYSTEM MUST BE STAMPED BY A LICENSED CIVIL REGISTER AND SUBMITTED TO ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION.

SECTION 6 - STRUCTURAL STEEL AND MISCELLANEOUS METALS

STEEL SHALL CONFORM TO ASTM A992 LINLESS OTHERWISE NOTED. SOLIARE OR STEEL SHALL CONFORM TO ASTM A992 UNLESS OTHERWISE NOTED. SQUARE OR RECTANGULAR STRUCTURAL STEEL TUBES SHALL CONFORM TO ASTM A500, GRADE B (FY=46KSI). ALL WORK SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS". SHOP DRAWINGS SHALL BE SUBMITTED FOR THE OWNER'S REPRESENTATIVES REVIEW BEFORE COMMENCING FABRICATION, SHOP DRAWINGS SHALL EVALUATED THE DIDENTIFY AND ASSOCIATION. SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY STRUCTURAL WELDING CODE, AWS D1.1, ALL FIELD WELDING TO BE ACCOMPLISHED BY AWS CERTIFIED WELDERS. ALL STEEL ANCHORS, TIES AND OTHER MEMBERS TO BE EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED. ALL MACHINE BOLTS SHALL BE ASTM A307 U.N.O. AND SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF LOCKING NUTS, ALL NUTS, BOLTS, WASHERS AND MISC. STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED.

°SECTION 7 - JOB SAFETY

THE ENGINEER HAS NOT BEEN RETAINED NOR COMPENSATED TO PROVIDE DESIGN AND/OR CONSTRUCTION REVIEW SERVICES RELATED TO THE CONTRACTOR'S SAFETY PRECAUTIONS OR TO MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES FOR PRECADITIONS OR 10 MEANS, ME HOUS, 1ECHNIQUES, SEQUENCES OR PROCEDURI THE CONTRACTOR TO PERFORM HIS WORK. THE UNDERTAKING OF PERIODIC SITE VISITS BY THE ENGINEER SHALL NOT BE CONSTRUED AS SUPERVISION OF ACTUAL CONSTRUCTION NOR MAKE HIM RESPONSIBLE FOR PROVIDING A SAFE PLACE FOR THE PROPERTY OF THE PROPERTY OF THE PROVIDING A SAFE PLACE FOR THE PROVIDING A SAFE PLACE THE PERFORMANCE OF WORK BY THE CONTRACTOR, SUBCONTRACTORS, SUPPLIERS OR THEIR EMPLOYEES, OR FOR ACCESS, VISITS, USE WORK, OR OCCUPANCY BY ANY

AMPED AND SIGNED THEY RELIMINARY NOT FOR THESE DRAWINGS ARE: LOCATION SPECIFIED. IF R LOCATION PLEASE STOM DESIGNS.

Designs

10B #

Main floor = 1652 sf

Foundation = 1557 st Basement = 1378 st

Front Porch = 50 sf

Garage = 881 s

Basement finished = 1262 sf

SCALE:

SHEET:

Contractors/Sub-Contractors to verify all finish work with contract. Plans as drawn may represent future options and possible finished layout. Extent of Finish as dictated by Sales Contract supersedes drawing representations.

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Crookston

DATE3/26/2019
4/17/2019
4/19/2019 ction Set

DESCRIPTION:
Home (4767 Thur
Mountain Bend
, Idaho

Custom

19011

1'' = 50'



SHEAR WALL SCHEDULE				
TYPE	EDGE NAIL SPACING	FIELD NAIL SPACING	EDGE STAPLE SPACING	FIELD STAPLE SPACING
A	6"	12"	3"	12"
В	4"	12"	2"	12"
	3"	12"	NOT PERMITTED	NOT PERMITTED
	2"	12"	NOT PERMITTED	NOT PERMITTED
NOTES:				

1) SHEETING NAILS ARE TO BE

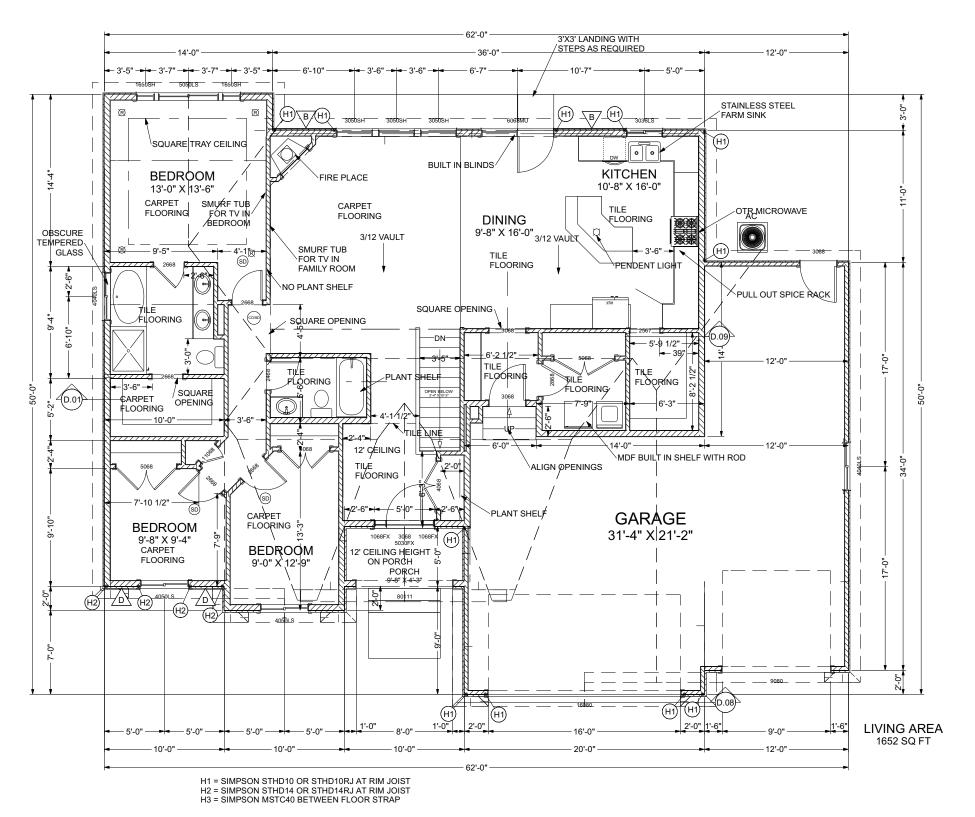
8d COMMON NAILS
2) ALL PANEL EDGES ARE TO HAVE

BLOCKING WITH EDGE NAILING
3) TYPE C AND D WALLS ARE TO HAVE 3"
NOMINAL FRAMING AT ALL PANEL EDGES.

4) UNLESS NOTED OTHERWISE SHEET ALL EXTERIOR WALLS AS TYPE "A" WALLS
5) STAPLES SHALL BE 16 GAGE AND HAVE A

MINIMUM CROWN WIDTH OF 7/16" AND SHALL BE INSTALLED WITH THEIR CROWN PARALLEL TO THE LONG DIMENSION OF THE FRAMING MEMBERS.

6) STAPLES SHALL HAVE 1" MINIMUM PENETRATION



MAIN FLOOR PLAN

Crookston

	_	_		
DATE	3/26/2019	4/17/2019	4/19/2019	
ВУ	BJC	BJC	BJC	
DESCRIPTION	Construction Set	Construction Set	Construction Set	
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	_	Construction Set	BJC
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1.7	~	Construction Set	BJC

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DRAWINGS PROVIDED BY:
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JOB #

Main floor = 1652 sf Foundation = 1557 sf Basement = 1378 sf

Garage = 881 sf

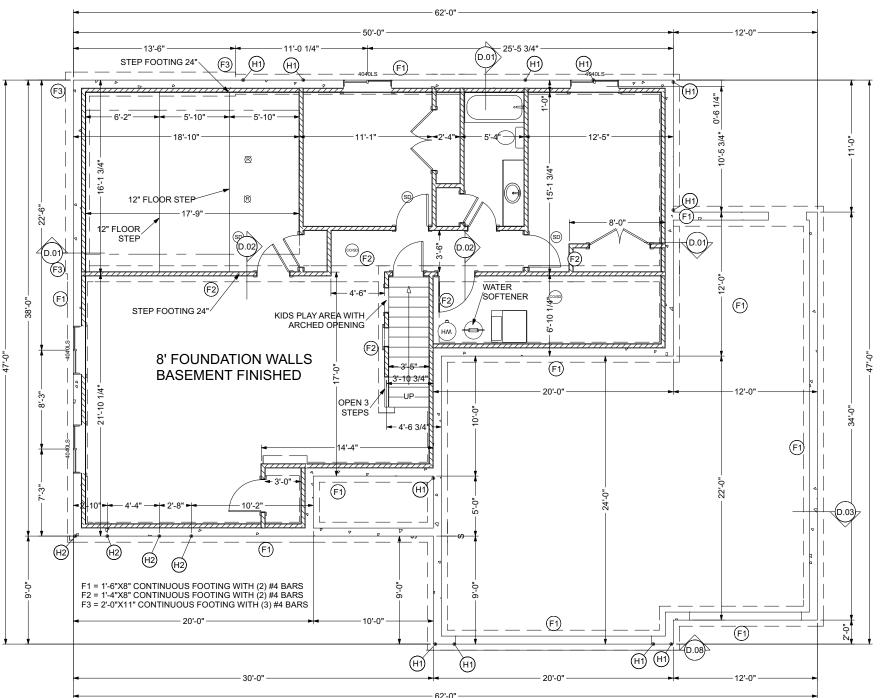
Basement finished = 1262 sf Front Porch = 50 sf

19011

SCALE:

1/4" = 1'

SHEET:



FOUNDATION PLAN

H1 = SIMPSON STHD10 OR STHD10RJ AT RIM JOIST H2 = SIMPSON STHD14 OR STHD14RJ AT RIM JOIST H3 = SIMPSON MSTC40 BETWEEN FLOOR STRAP

Main floor = 1652 sf Foundation = 1557 sf Basement = 1378 sf Basement finished = 1262 sf Front Porch = 50 sf

> SCALE: SHEET:

Garage = 881 sf

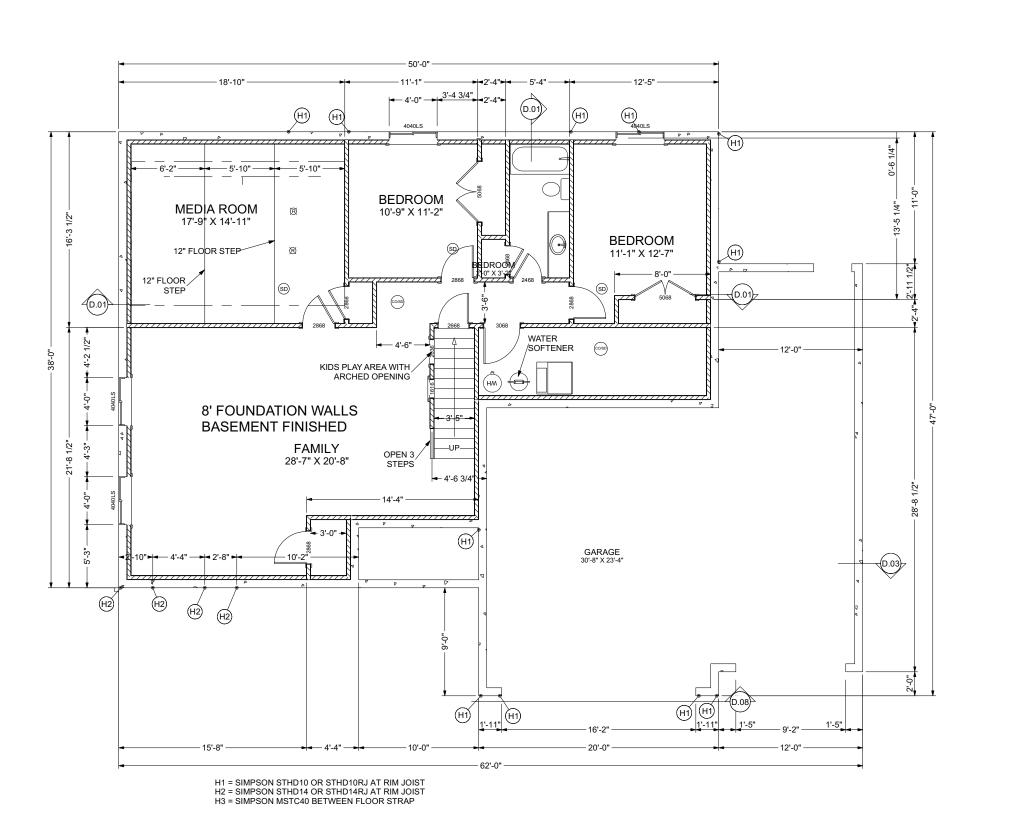
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1/4" = 1'

19011

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DRAWINGS PROVIDED BY:
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Main floor = 1652 sf Foundation = 1557 sf Basement = 1378 sf Basement finished = 1262 sf Front Porch = 50 sf

Garage = 881 sf

19011

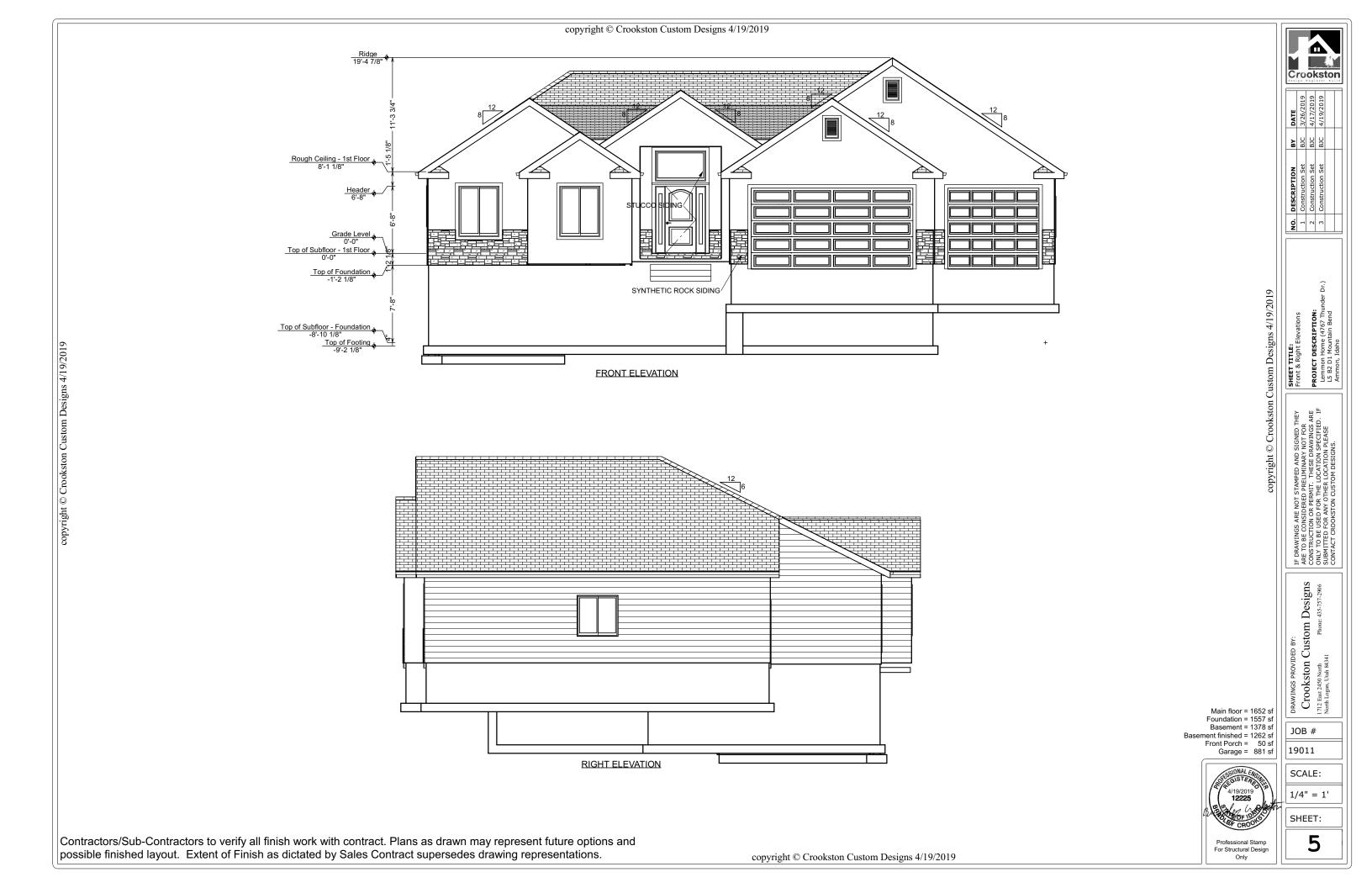
SCALE:

1/4" = 1'

SHEET:

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FINISHED BASEMENT AREA





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1/4" = 1'

SHEET:

essional Stamp tructural Design Only

PLAN SY	MBOLS
(W??)	WINDOW OPENING
700	DOOR OPENING
	DOUBLE SWING DOOR OPENING
	COOK RANGE WITH OVEN
REF.	REFRIGERATOR
	LAUNDRY WASHER & DRYER
DW	DISH WASHER
	WALL OVEN

00	DOUBLE KITCHEN SINK
•	SINGLE UTILITY SINK
	BATHROOM SINK
	PEDESTAL SINK
	TOILET
	BATH TUB WITH SHOWER
	LARGE SOAKER TUB
	SHOWER
d Si	WATER HEATER
	FORCED AIR FURNACE
	INSERT FIRE PLACE
	WOOD STOVE

FASTENING SCHEDULE

CONNECTION	FASTENING (a), (m)	LOCATION
1. Joist to sill or girder	3 - 8d common (2-1/2" × 0.131")	toenail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
2. Bridging to joist	2 - 8d common (2-1/2" × 0.131")	toenail each end
	2 - 3" × 0.131" nails	
	2 - 3" 14 gage staples	
3. 1" × 6" subfloor or less to each joist	2 - 8d common (2-1/2" × 0.131")	face nail
4. Wider than 1" × 6" subfloor to each joist	3 - 8d common (2-1/2" × 0.131")	face nail
5. 2" subfloor to joist or girder	2 - 16d common (3-1/2" × 0.162")	blind and face nail
6. Sole plate to joist or blocking	16d (3-1/2" × 0.135 ") at 16" o.c.	typical face nail
p-a jg	3" × 0.131" nails at 8" o.c.	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Sole plate to joist or blocking at braced	3" 14 gage staples at 12" o.c.	†
Wall panel	3- 16d (3-1/2" × 0.135") at 16" o.c.	braced wall panels
	4 - 3" × 0.131" nails at 16" o.c.	bracea wan pariois
	4 - 3" 14 gage staples at 16" o.c.	
7. Top plate to stud	2 - 16d common (3-1/2" × 0.162")	end nail
7. Top place to stud	3 - 3" × 0.131" nails	end naii
	3 - 3" 14 gage staples	
9. Ctud to cale plate	0 0 1	toenail
8. Stud to sole plate	4 - 8d common (2-1/2" × 0.131") 4 - 3" × 0.131" nails	toenaii
	3 - 3" 14 gage staples	
	2 - 16d common (31/2" × 0.162")	end nail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
9. Double studs	16d (3-1/2" × 0.135") at 24" o.c.	face nail
	3" × 0.131" nail at 8" o.c.	
	3" 14 gage staple at 8" o.c.	
10. Double top plates	16d (3-1/2" × 0.135") at 16" o.c.	typical face nail
	3" × 0.131" nail at 12" o.c.	
Double top plates	3" 14 gage staple at 12" o.c.	
Double top plates	8 - 16d common (3-1/2" × 0.162")	lap splice
	12 - 3" × 0.131" nails	
	12 - 3" 14 gage staples	
11. Blocking between joists or rafters to top	3 - 8d common (2-1/2" × 0.131")	toenail
plate	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
12. Rim joist to top plate	8d (2-1/2" × 0.131") at 6" o.c.	toenail
	3" × 0.131" nail at 6" o.c.	1
	3" 14 gage staple at 6" o.c.	
13. Top plates, laps and intersections	2 - 16d common (3-1/2" × 0.162")	face nail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	†
14. Continuous header, two pieces	16d common (3-1/2" × 0.162")	16" o.c. along edge
15. Ceiling joists to plate	3 - 8d common (2-1/2" × 0.131")	toenail
ro. Coming joints to plate	5 - 3" × 0.131" nails	tooriaii
	5 - 3" 14 gage staples	+
16. Continuous header to stud	4 - 8d common (2-1/2" × 0.131")	toenail
17. Ceiling joists, laps over partitions	3 - 16d common (3-1/2" × 0.162") min.,	face nail
(see Section 2308.10.4.1, Table 2308.10.4.1)	Table 2308.10.4.1	lace hall
(300 00011011 2300.10.4.1, Table 2000.10.4.1)		
	4 - 3" × 0.131" nails	1
40 Ocilian ininta ta manali I	4 - 3" 14 gage staples	f
18. Ceiling joists to parallel rafters	3 - 16d common (3-1/2" × 0.162") minimum,	race nall
(see Section 2308.10.4.1, Table 2308.10.4.1)	Table 2308.10.4.1	1
	4 - 3" × 0.131" nails	1
	4 - 3" 14 gage staples	
19. Rafter to plate	3 - 8d common (2-1/2" × 0.131")	toenail
	3 - 3" × 0.131" nails	1
(see Section 2308.10.1, Table 2308.10.1)	3 - 3" 14 gage staples	

CONNECTION	FASTENING (a), (m)	LOCATION
20. 1" diagonal brace to each stud and plate	2 - 8d common (2-1/2" × 0.131")	face nail
	2 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
21. 1" × 8" sheathing to each bearing	3 - 8d common (2-1/2" × 0.131")	face nail
22. Wider than 1" × 8" sheathing to each bearing	3 - 8d common (2-1/2" × 0.131")	face nail
23. Built-up corner studs	16d common (3-1/2" × 0.162")	24" o.c.
•	3" × 0.131" nails	16" o.c.
	3" 14 gage staples	16" o.c.
24. Built-up girder and beams		face nail at top and bottom staggered on opposite sides
3	3" × 0.131" nail at 24" o.c.	33
	3" 14 gage staple at 24" o.c.	
	2 - 20d common (4" × 0.192")	face nail at ends and at each splice
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
25. 2" planks	16d common (3-1/2" × 0.162")	at each bearing
26. Collar tie to rafter	3 - 10d common (3" × 0.148")	face nail
20. Cond. do to ranto.	4 - 3" × 0.131" nails	1400 11411
	4 - 3" 14 gage staples	
27. Jack rafter to hip	3 - 10d common (3" × 0.148")	toenail
27. Back raiter to hip	4 - 3" × 0.131" nails	toeriaii
	4 - 3" 14 gage staples	
	2 - 16d common (3-1/2" × 0.162")	face pail
	3 - 3" × 0.131" nails	lace Itali
	3 - 3" 14 gage staples	
00 Dfft- 0 h		41
28. Roof rafter to 2-by ridge beam	2 - 16d common (3-1/2" × 0.162")	toenail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
	2 -16d common (3-1/2" × 0.162")	face nail
	3 - 3" × 0.131" nails	
	3 - 3" 14 gage staples	
29. Joist to band joist	3 - 16d common (3-1/2" × 0.162")	face nail
	4 - 3" × 0.131" nails	
	4 - 3" 14 gage staples	
30. Ledger strip	3 - 16d common (3-1/2" × 0.162")	face nail at each joist
	4 - 3" × 0.131" nails	
	4 - 3" 14 gage staples	
31. Wood structural panels and particleboard (b)	1/2" and less	6d (c), 1
Subfloor, roof and wall sheathing (to framing)		2-3/8" × 0.113" nail (n)
		1-3/4" 16 gage (o)
	19/32" to3/4"	8d (d) or 6d (e)
		2-3/8" × 0.113" nail (p)
		2" 16 gage staple (p)
Single floor (combination subfloor-underlayment to		2 To gage staple (p)
framing)		
··-···································	7/8" to 1"	8d (c)
	11/8" to 11/4"	10d (d) or 8d (e)
	3/4" and less	6d (e)
	7/8" to 1"	8d (e)
	11/8" to 11/4"	10d (d) or 8d (e)
32. Panel siding (to framing)	1/2" or less	6d (f)
	5/8"	8d (f)
33. Fiberboard sheathing (g)	1/2"	No. 11 gage roofing nail (h)
		6d common nail (2" × 0.113")
		No. 16 gage staple (i)
	25/32"	No. 11 gage roofing nail (h)
		8d common nail (2-1/2" × 0.131")
		No. 16 gage staple (i)
24 Interior penaling	1/4"	
34. Interior paneling	1/4"	4d (j)
	3/8"	6d (k)

a. Common or box nails are permitted to be used except where otherwise stated.

Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Section 2305. Nails for wall sheathing are permitted to be common, box or casing.

- Common or deformed shank (6d 2" × 0.113"; 8d 21/2" × 0.131"; 10d 3" × 0.148").
- I. Common (6d 2" × 0.113"; 8d 21/2" × 0.131"; 10d 3" × 0.148").
- . Deformed shank (6d 2" × 0.113"; 8d 21/2" × 0.131"; 10d 3" × 0.148").
- f. Corrosion-resistant siding (6d 17/8" × 0.106"; 8d 23/8" × 0.128") or casing (6d 2" × 0.099"; 8d 21/2" × 0.113") nail
- Fasteners spaced 3 inches on center at exterior edges and 6 inches on center at intermediate supports, when used as structural sheathing. Spacing shall be 6 inches on center on the edges and 12 inches on center at intermediate supports for nonstructural
- Corrosion-resistant roofing nails with 7/16-inch-diameter head and 1-1/2-inch length for 1/2-inch sheathing and 1-3/4-inch length for 25/32-

i. Corrosion-resistant staples with nominal 7/16-inch crown or 1-inch crown and 1-1/4-inch length for 1/2-inch sheathing and 1-inch length for 25/32-inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwise

- Casing (1-1/2" × 0.080") or finish (1-1/2" × 0.072") nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
- k. Panel supports at 24 inches. Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
- I. For roof sheathing applications, 8d nails (21/2" × 0.113") are the minimum required for wood structural panels.
- Staples shall have a minimum crown width of 7/16 inch.
- n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.
- o. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports for subfloor and wall sheathing and 3 inches on center at edges, 6 inches at intermediate supports for roof sheathing.
- p. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports

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Crookston Custom Designs
1712 East 2450 North Phone: 435-757-2906
North Logan, Usiah 84341

19011

SCALE: 1/4" = 1'

SHEET:

Main floor = 1652 sf

Foundation = 1557 sf

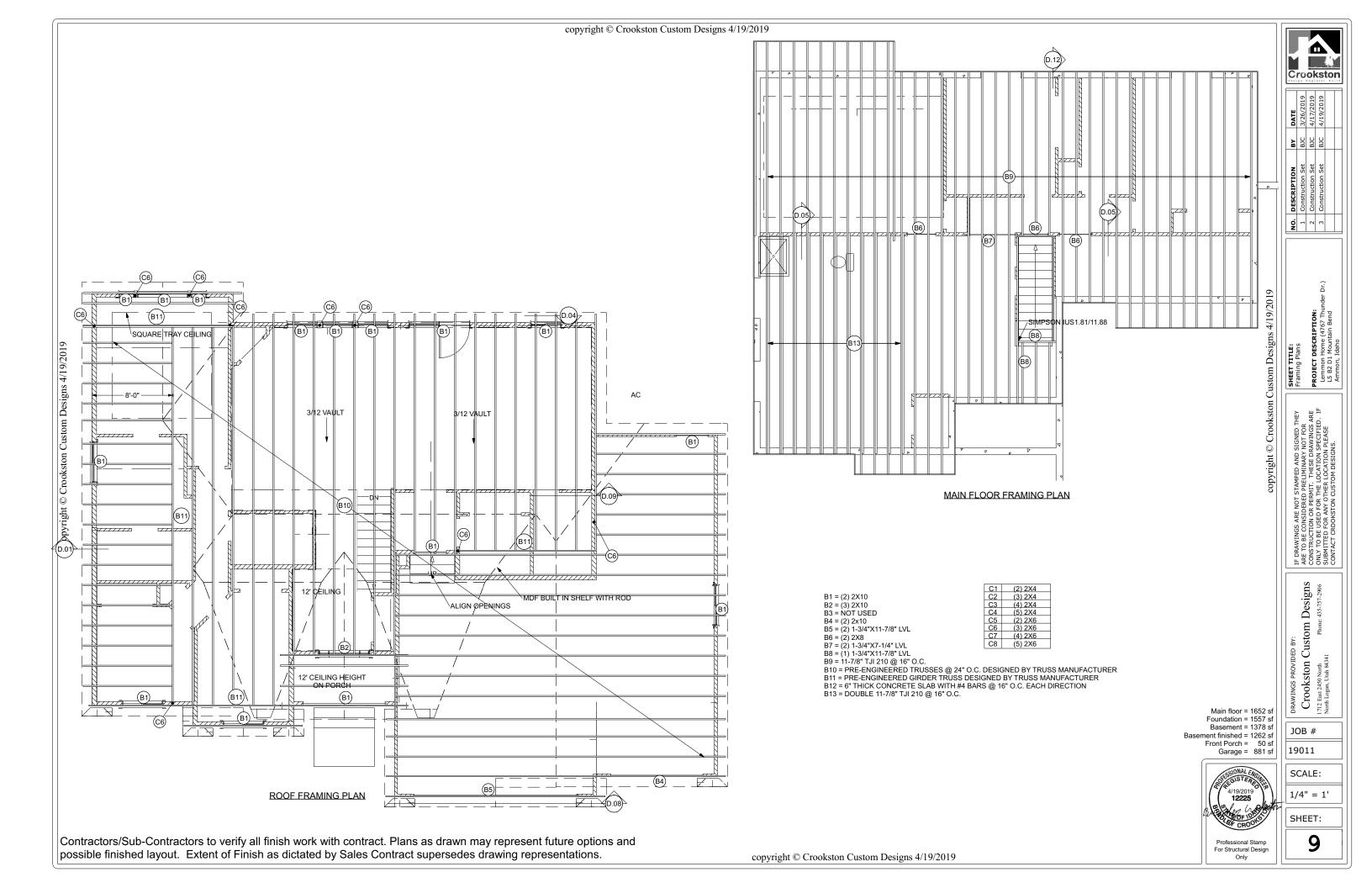
Basement = 1378 sf

Front Porch = 50 sf

Garage = 881 sf

Basement finished = 1262 sf

Contractors/Sub-Contractors to verify all finish work with contract. Plans as drawn may represent future options and possible finished layout. Extent of Finish as dictated by Sales Contract supersedes drawing representations.



- 30'-0" - 30'-0"-MAXIMUN SPEC HOME (4767 Thunder Dr.) L5 B2 D1 Mountain Bend Ammon, Idaho Zoning: R1 Residence

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Crookston Custom Designs
1712 East 2450 North Phone: 435-757-2906
North Logan, Uniah 98341

Main floor = 1652 sf Foundation = 1557 sf Basement = 1378 sf Basement finished = 1262 sf Front Porch = 50 sf Garage = 881 sf

19011

SCALE: 1" = 10'

SHEET:

THUNDER DRIVE